

Leydens Wholesalers & Distributors Dublin, No. 158A

Infrastructure Design Report

210178-DBFL-Z0-XX-RP-C-0001

INFRASTRUCTURE



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1 INTRODUCTION

1.1 Background

DBFL Consulting Engineers were commissioned to provide engineering services to accompany a planning submission for a proposed mixed-use development at Leydens Wholesalers & Distributors Dublin, No. 158A, Dublin 3, with Dublin City Council.

1.2 Objectives

This report addresses the main infrastructure elements including;

- Site access;
- Proposed roads;
- Management of surface water runoff (Surface Water Management Plan);
- Foul drainage strategy;
- Water supply;

1.3 Development Proposals

Malkey Limited intend to apply for permission for development (Large-scale Residential Development (LRD)) at this c. 0.55 hectare site at the former Leydens Wholesalers & Distributors, No. 158A Richmond Road, Dublin 3, D03 YK12. The site is bounded to the north-east by Richmond Road, to the west/south-west by No. 146A and Nos. 148-148A Richmond Road (pending application ABP Reg. Ref. TA29N.312352), to the south/south-west by a residential and commercial development (Distillery Lofts) and to the east/south-east by the Former Distillery Warehouse (derelict brick and stone building). Improvement works to Richmond Road are also proposed including carriageway widening up to c. 6 metres in width, the addition of a c. 1.5 metre wide one-way cycle track/lane in both directions, the widening of the northern footpath on Richmond Road to a minimum of c. 1.8 metres and the widening of the southern footpath along the site frontage which varies from c. 2.2 metres to c. 7.87 metres, in addition to a new signal controlled pedestrian crossing facility, all on an area of c. 0.28 hectares. The development site area and road works area will provide a total application site area of c. 0.83 hectares.

The proposed development will principally consist of: a Large-scale Residential Development (LRD) comprising the demolition of existing industrial structures on site (c. 3,359 sq m) and the construction of a mixed-use development including artist studios (c. 749 sq m), a creche (c. 156 sq



m), a retail unit (c. 335 sq m), and a gym (c. 262 sq m), and 133 No. residential units (65 No. one bed apartments and 68 No. two bed apartments). The development will be provided in 3 No. blocks ranging in height from part 1 No. to part 10 No. storeys as follows: Block A will be part 1 No. storey to part 4 No. storeys in height, Block B will be part 1 No. storeys to part 10 No. storeys in height (including podium) and Block C will be part 1 No. storeys to part 9 No. storeys in height (including podium). The proposed development has a gross floor area of c. 14,590 sq m and a gross floor space of c. 13,715 sq m.

The development also proposes the construction of: a new c. 204 No. metre long flood wall along the western, southern and south-eastern boundaries of the proposed development with a top of wall level of c. 6.4 metres AOD to c. 7.15 metres AOD (typically c. 1.25 metres to c. 2.3 metres in height) if required; and new telecommunications infrastructure at roof level of Block B including shrouds, antennas and microwave link dishes (18 No. antennas enclosed in 9 No. shrouds and 6 No. transmission dishes, together with all associated equipment) if required. A flood wall and telecommunications infrastructure are also proposed in the adjoining Strategic Housing Development (SHD) application (pending decision ABP Reg. Ref. TA29N.312352) under the control of the Applicant. If that SHD application is granted and first implemented, no flood wall or telecommunications infrastructure will be required under this application for LRD permission (with soft landscaping provided instead of the flood wall). If the SHD application is refused permission or not first implemented, the proposed flood wall and telecommunications infrastructure in the LRD application will be constructed.

The proposed development also provides ancillary residential amenities and facilities; 25 No. car parking spaces including 13 No. electric vehicle parking spaces, 2 No. mobility impaired spaces and 3 No. car share spaces; 2 No. loading bays; bicycle parking spaces; motorcycle parking spaces; electric scooter storage; balconies and terraces facing all directions; public and communal open space; hard and soft landscaping; roof gardens; green roofs; boundary treatments; lighting; ESB substation; switchroom; meter room; comms rooms; generator; stores; plant; lift overruns; and all associated works above and below ground.

1.4 Site Location and Topography and Characteristics

The site has an area of circa 0.55ha, excluding the Richmond Road upgrade works and is accessed from Richmond Road. The subject brownfield site, previously occupied by Leydens Wholesalers & Distributors Dublin, is bound to the west and southwest by a site upon which a recently submitted SHD application has been made to An Bord Pleanála (ABP). To the north lies the Richmond Road

corridor. The subject site is bound to the south-east by Distillery Lofts commercial premises whilst to the east lies the remains of a previous commercial building which is in a state of disrepair. The site location is shown in *Figure 1-1* below.

A section of Richmond Road, which provides access to the subject site, is proposed to be upgraded as part of the development and includes cycle tracks/lanes and footpaths on both sides of the road.

The site is generally flat, with a fall from Richmond Road at the north-west to the south-east, at an average gradient of 1/130. The industrial site is fully paved with concrete and asphalt surfacing.



Figure 1-1 Site Location Map [Google Maps]

The site is within the jurisdiction of the Dublin City Development Plan, 2022-2028. Based on the information provided on the Map E, "Use Zonings Objectives" map, the subject site is zoned Z10 in the 2022 – 2028 Dublin City Development Plan *"to consolidate and facilitate the development of inner city and inner suburban sites for mixed-uses"*.

2 ROADS AND ACCESS

2.1 Vehicular Access

The subject development will benefit from direct vehicular access onto Richmond Road as presented in Figure 2-1. The proposed site access will be located to the west of the Lofts and the Stables Apartment complex access. The access will take the form of a priority-controlled junction and access is proposed to be secured by a proposed gate to restrict access to permitted residents / visitors only. The design of the new access junction, in addition to the internal road, has been actively influenced by and subsequently complies with DMURS.



Figure 2-1 Site access arrangement

Further details of the site access arrangements has been illustrated in DBFL Drawing No. 210178-DBFL-TR-SP-DR-C-1102. Access arrangements are shown for the proposed development after the road upgrade works have been completed, tying into the existing Richmond Road alignment as well as the proposed road upgrade tying into the post DCC Richmond Road enhancement corridor objective works have been complete. This approach would ensure the proposed Richmond Road upgrade works aligns with the future DCC plans to upgrade the Richmond Road corridor on either side of the proposed works.

2.2 Richmond Road Works

The scheme also includes for enhancements of approx. 204m of the Richmond Road corridor comprising improved footways and the introduction of dedicated cycle infrastructure. The provision of dedicated high-quality pedestrian footways (1.8m to 2.0m wide) and cycle tracks/lanes

(1.5m wide) which will be provided as part of the Richmond Road upgrades proposed as part of the subject application. A signalised pedestrian crossing is proposed approx. 40m north-west of Block A. It is noted that these upgrades were initially incorporated within the adjoining SHD development (ABP PI. Ref. 312352) but has now been included within the subject application in order that these road upgrades are independent of the SHD development (i.e., will be delivered regardless of the SHD development' planning application outcome). The proposed Richmond Road works ties into the existing road with gradual tapers. The north-west tie-in is proposed as a temporary tie-in until the roadworks are extended north at a later stage. Refer to Figure 2-2 for the extent of the proposed Richmond Road works.



Figure 2-2 Richmond Road Upgrade Proposed Works

Refer to Figure 2-3 for the Richmond Road works when the road upgrade is extended north at a later stage.



Figure 2-3 Development Frontage Arrangements Post DCC Richmond Road Corridor Enhancement Objective

The proposed cross-section for Richmond Road is shown on the enclosed drawing 210178-DBFL-RD-SP-DR-C-5202 and the road long sections on drawings 210178-DBFL-RD-SP-DR-C-3201 to 3203. The proposed Richmond Road works tie in at existing levels north of Richmond Road (across the road from the proposed development) to ensure minimal change to the exiting levels at property accesses.

On the northern side of Richmond Road, a 60mm kerb is proposed between the road and the off-road cycle track and a 60mm kerb between the cycle track and the footpaths.

On the southern side (Development side) of Richmond Road, a 100mm kerb is proposed between the road and the off-road cycle track and a 50mm kerb between the cycle track and the footpaths.

The existing alignment is to be kept as far as possible with minimal changes to existing vertical alignment proposed at centreline.

A Traffic & Transportation Assessment (TTA), prepared by DBFL Consulting Engineers, is included as a standalone document and addresses car parking, cycle parking facilities and access to public transport.

The development is designed in accordance with DMURS (*Design Manual Urban Roads and Streets*) and a DMURS Compliance Statement is included in the separate TTA report included in this submission.

2.3 Vehicle Tracking

DBFL Drawing No. 210178-DBFL-TR-SP-DR-C-1103 indicates all vehicle tracking for a refuse vehicle, delivery goods vehicle and fire tender. Appropriate vehicle sizes are used for the tracking as indicated on drawing number 210178-DBFL-TR-SP-DR-C-1103. Designated on-site areas have been

provided for the refuse vehicle and delivery vehicle at the south-east corner of the site as indicated in Figure 2-4 below.

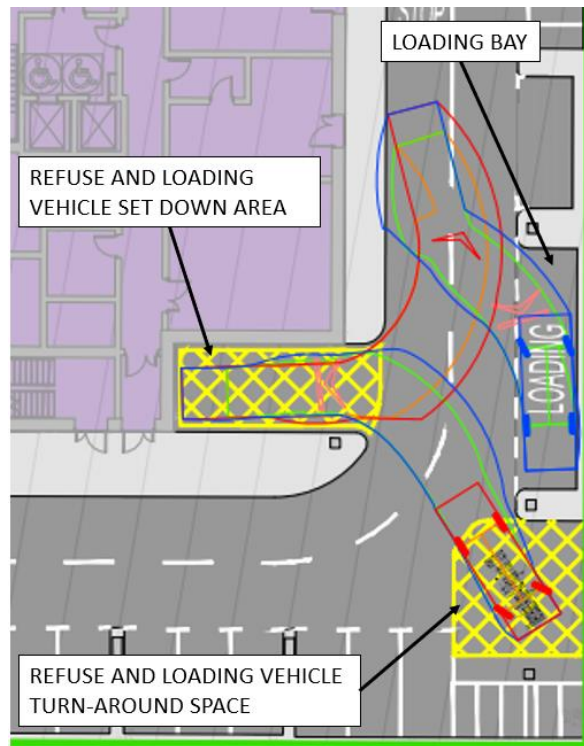


Figure 2-4 Refuse/delivery vehicle set down area

A fire tender route has been provided between Blocks A & B as indicated in Figure 2-5 below.

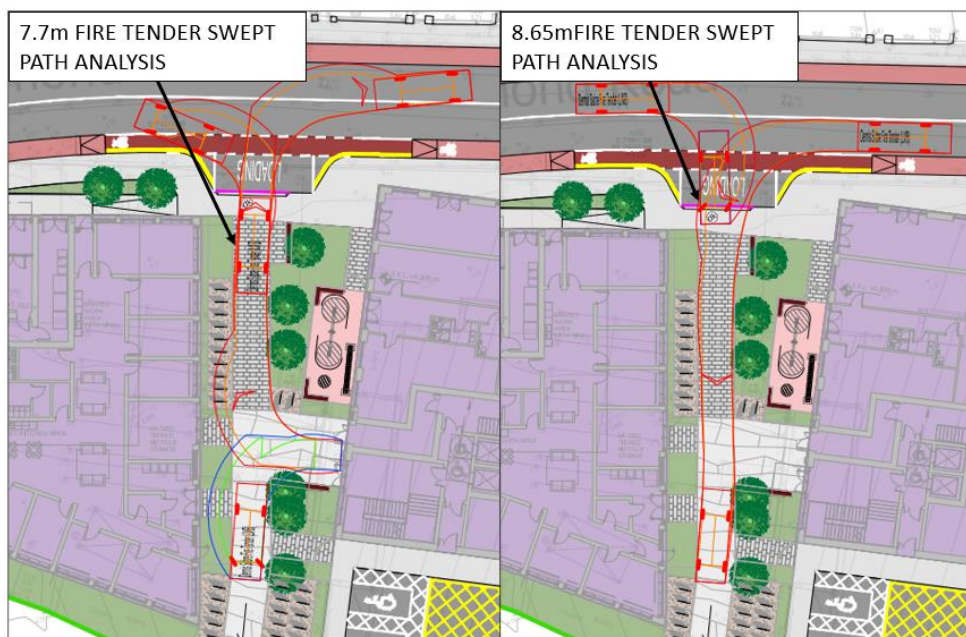


Figure 2-5 Fire tender access route



Further details on the site access is included in the separate TTA document and the enclosed drawing 210178-DBFL-TR-SP-DR-C-1103.

2.4 Available Sightlines

Sightlines at the entrance to the proposed development in accordance with the recommendations of DMURS are included on DBFL drawing no. 210178-DBFL-RD-SP-DR-C-1200.

2.5 Vehicle and Cycle Parking

An appropriate amount of vehicle and cycle parking has been provided as set out in the separate TTA document.



3 EXISTING UTILITIES

Existing utilities within the site and in the immediate vicinity of the site was determined by as-built records from utility companies and a GPR Utility Survey completed by Murphy Geospatial.

Existing utilities as identified by the GPR Survey include:

- ESB (Electrical) - ESB records show electrical connections to Ebox offsite. None found on site;
- Public lighting - Connections to the lamp poles were identified on site. Two lamp posts were reported as being disused – please refer to the attached Murphy Geospatial Survey attached as **Appendix I**;
- EIR – None identified on site;
- UPC - – None identified on site;
- ENET - – None identified on site;
- GNI (Gas) – Partially located on site. Refer to drawings in report;

Refer to Murphy Geospatial Utility Survey included in **Appendix I** for further details.



4 SITE INVESTIGATION

A preliminary site investigation was conducted as attached in **Appendix J** and includes the following tests:

- 3 x Trial pits up to 1.5m depth
- 2 x infiltration tests

The ground investigation revealed that the site is surfaced with 10mm layer of asphalt. Below the surfacing, two distinctive soil layers have been identified:

Made Ground: Greyish brown sandy clayey angular fine to coarse GRAVEL BACKFILL. Sand is fine to coarse.

In-situ Material: Light brown sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is subangular to subrounded fine to medium.

5 SURFACE WATER MANAGEMENT PLAN

5.1 Existing Surface Water Drainage

Local Authority record drawings indicate a large diameter surface water sewer in the vicinity of the site. There is a 1350mm diameter surface water sewer at the proposed entrance to the subject development under Richmond Road. Refer to *Figure 5-1* below.

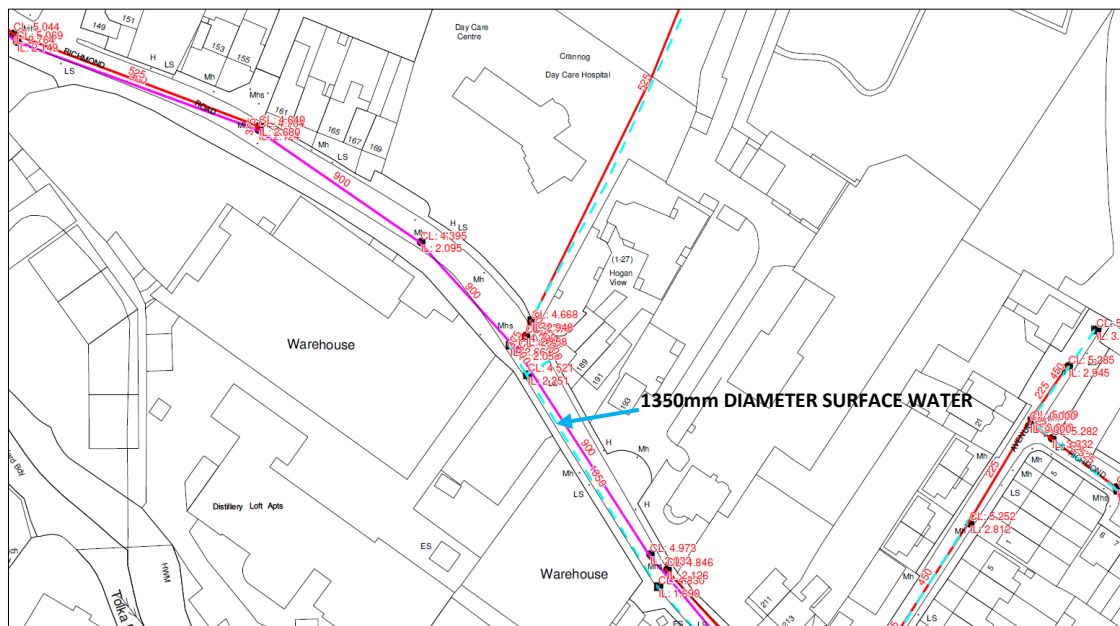


Figure 5-1 Extract of Local Authority Record Drawings

Within the site, the existing surface water network comprises of a combination of gullies, concrete channels, 100mm - 225mm diameter uPVC surface water pipes, which collects surface water runoff from the existing site, and discharges unattenuated runoff to the public surface water sewer in Richmond Road near the current entrance of the site.

5.2 General Surface Water Management Policy

Dublin City Council is within the Greater Dublin Area (GDA) as identified in the Greater Dublin Strategic Drainage Study (GDSDS). The GDSDS outlines regional drainage policies to address the drainage needs of the GDA. These policies address surface water management from development sites, from the point of view of water quality, quantity, risk of flooding and compliance with relevant environmental legislation. As outlined in the GDSDS, proposed developments must be drained on separate foul and surface water drainage systems and must incorporate Sustainable Urban Drainage Systems (SuDS) for the management of surface water runoff. The SuDS strategy follows CIRIA SuDS Guidelines.



The guidelines require the following 4 main criteria to be provided by the design;

- Criterion 1: River Water Quality Protection – River Water Quality Protection – Satisfied by providing interception storage and treatment of surface water run-off by SUDS features such as permeable paving, tree pits, green/blue roof/podium & filter layers under attenuation.
- Criterion 2: River Regime Protection – Satisfied by attenuating surface water run-off in association with flow control devices prior to discharge off site at greenfield runoff rate within the green/blue roof and cellular below ground storage.
- Criterion 3: Level of Service (flooding) for the site – satisfied by the development's surface water drainage design, planned flood routing and on-site storage. Also refer to DBFL Site Specific Flood Risk Assessment.
- Criterion 4: River flood protection – Satisfied by attenuating surface water discharge to greenfield runoff rates, addressing pluvial flood risk associated with the 1 in 100 year storm and avoiding development the addition of a flood wall in the proposals to protect the development from river flooding.

5.3 Management of Surface Water Runoff

To manage surface water runoff from the development, it is proposed to discharge attenuated runoff from the development to the existing public surface water sewer at the south east corner of the site in Richmond Road. Surface water storage will be provided within the site to accommodate runoff from a 1% AEP event plus 20% climate change. A combination of SuDS (sustainable urban drainage) features and traditional drainage, such as gullies and pipes will be utilised to manage runoff from the site.

Two separate surface water management systems have been identified as part of the proposed works to include:

1. The proposed development
2. The area of proposed Richmond Road works which would provide access to the proposed development.



5.4 Surface Water management - Proposed development (Excluding the Richmond Road works)

To control the quality and quantity of runoff from the site it is proposed to incorporate significant SuDS measures within the scheme, including a green/blue roof system for the terrace and building roofs and permeable paving for hard surfaces where possible.

Surface water runoff not absorbed by SuDS features will be attenuated to Q_{bar} (Greenfield Runoff) with runoff exceeding this stored on site for up to a 1% AEP (Annual Exceedance Probability), in accordance with the requirements of the GDSDS (Greater Dublin Strategic Drainage Study). Surface water runoff from the building roofs is contained within the green/blue roof system within an underground cellular storage located in the south-east corner of the site, under the proposed road. Further ground level storage is also provided in the form of permeable paving, designed with enough storage to contain the 1% AEP storm event for the area between blocks A and B. Any incidental surface water build-up in this area would drain north to Richmond Road or into the proposed surface water sewer via the linear drains at the entrances to the buildings.

For severe (>1%AEP) storm events, an overland surface water strategy has been developed to ensure buildings are not flooded in the case of these storm events and appropriate freeboard has been allowed for. It is intended that all surface water collected will pass through an appropriate petrol interceptor and grease trap.

5.4.1 Surface Water Attenuation

Surface water runoff from the development is designed to be attenuated to greenfield runoff (Q_{bar}), in accordance with the recommendations of the GDSDS. Surface water run-off from the surface water catchment will be controlled using a vortex flow control device (Hydrobrake or equivalent) on the surface water outlet from the catchment area. Q_{bar} has been calculated based on the effective drainage area for the scheme, which is calculated as circa 0.471ha (effective catchment area). Refer to dr. 210178-DBFL-CS-SP-DR-C-1310 and Figure 5-2 for the extent of the effective drainage area.

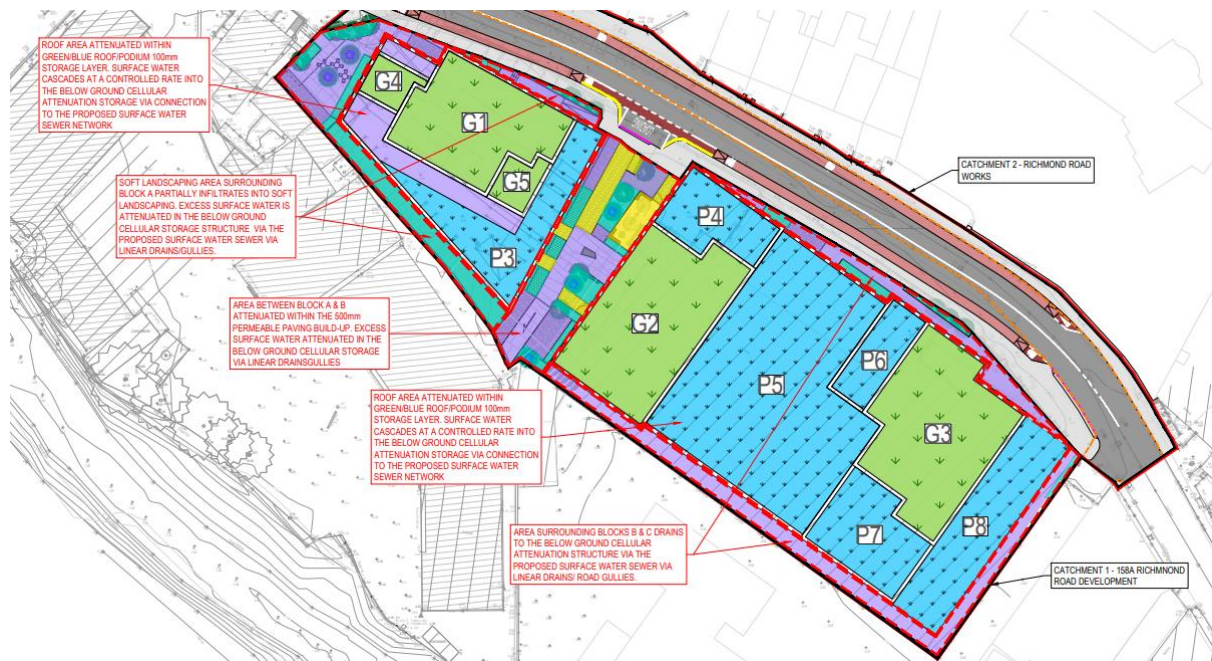


Figure 5-2 Proposed development catchment area

Q_{bar} is calculated using the *Institute of Hydrology* equation, as recommended in the Greater Dublin Strategic Drainage Study (GSDS), as follows:

$$Q_{bar [rural]} = 0.00108 \times AREA^{0.89} \times SAAR^{1.17} \times Soil^{2.17}$$

Where:

- $Q_{bar[rural]}$ is the mean catchment annual flow from a rural catchment in m^3/s ;
- AREA is the area of the catchment in km^2 . For a catchment area less than 50ha, calculate Q_{bar} for 50 ha and pro rata it. Area = 50ha or $0.5km^2$
- SAAR is the standard average annual rainfall = 729mm.
- SOIL is the soil index, with 5 soil types used and SPR values (standard percentage runoff) applied to each soil type.

The SPR values for the 5 soil types are as follows:

Soil 1 = 0.1; Soil 2 = 0.3; Soil 3 = 0.37; Soil 4 = 0.47; Soil 5 = 0.53;

A soil value of 0.37 (**Soil Type 3**) is applied for the subject site as derived from the preliminary site investigation infiltration results. The infiltration coefficient derived from the infiltration tests and used to calculate the appropriate storage volumes for SuDS elements such as permeable paving is 0.169m/h.



$$Q_{bar [rural]} = 0.00108 \times (0.050 \times 0.01)^{0.89} \times 729^{1.17} \times 0.37^{2.17} \times 1000/50$$

$$= 3.0l/s/ha$$

= 1.6 l/s for the gross catchment Area of 0.536 ha

Refer to **Appendix A** for the applicable Met Eireann Rainfall data.

AS RECOMMENDED IN THE GSDS, TO REDUCE THE RISK OF BLOCKAGES IN THE ORIFICE, THE ALLOWABLE OUTFLOW FROM THE SITE WILL BE LIMITED TO 2.0l/s.

5.4.2 Surface Water Storage

The overall required storage volumes for the catchment area has been calculated by modelling how the surface water cascades from the roof level storage to the cellular underground storage tank at a controlled rate (0.2l/s), and is then discharged into the public surface water sewer at another controlled flow rate ($Q_{bar} = 2l/s$). Permeable paving at ground level also acts as off line storage for surface water at ground level to reduce surface water entering the proposed surface water sewer system by encouraging infiltrating into the ground. The *Micro Drainage Source Control Cascade* module was used to model this process and calculations are attached in **Appendix B**. Figure 5-3 below depicts the cascading of surface water from one storage structure to the next before being discharged into the public surface water sewer in Richmond Road at a controlled rate.



Figure 5-3 Surface Water Storage Cascade



Roof Level Storage

The ABG Green/Bluroof System D108mm or similar approved is proposed for the roofs and terraces. A storage layer in the build-up will accommodate runoff from roofs and terraces. The storage provided in each green/blue roof and terrace is indicated Table 5-1. The green/blue roofs and terraces have been modelled to release attenuated surface water at a rate of 0.2l/s. Surface water would then cascade to the downstream cellular attenuation tank. An overflow structure would be installed in the outlet of the green/blue roofs and terraces in case of a roof outlet control blockage. Refer to **Appendix H** for ABG blue roof brochure.

Ground Level Storage

At ground level, surface water is attenuated in the proposed 100m² permeable paving build-up and in an underground cellular attenuation system. Storage is provided for runoff volumes for rainfall events up to a 1% AEP (Annual Exceedance Probability) plus 20% climate change. Surface water runoff at ground level between blocks A and B is stored off line within the permeable paving build-up and is sized to store the 1% AEP storm event of the catchment area between blocks A and B (0.048ha). Refer to Appendix B for the Micro Drainage Source Control calculations used to confirm the storage capacity of the permeable paving. 120m² Of permeable paving provides 18m³ of storage, assuming 30% porosity as per the CIRIA, document No. C521.

Additional storage of 114m³ is provided in an underground cellular attenuation system to attenuate surface water runoff from the entire site and would be the last storage structure within the attenuation train before surface water is released to the public surface water sewer at a controlled rate. The underground cellular storage tank is proposed at the south-east corner of the site under the proposed road, with attenuated flows discharging to existing public 1350mm surface water pipe in Richmond Road at the proposed entrance to the development. Refer to **Appendix G** for the proposed Stormbloc Brochure and Maintenance Procedures.



Table 5-1 Required and Provided Surface Water Storage

Total Surface Water Storage Provision			
Surface Type	Volume Required (m ³)	Storage Surface Area Provided (m ²)	Storage Volume Provided (m ³)
(P3) Green/Blue Podium	11.3	248.0	24
(P4) Green/Blue Podium	2.8	147.0	14
(P5) Green/Blue Podium	75.5	972.0	94
(P6) Green/Blue Podium	3.7	111.0	11
(P7) Green/Blue Podium	8.8	208.0	20
(P8) Green/Blue Podium	17.9	350.0	34
(G1) Green/Blue Roof	17.9	357.0	35
(G2) Green/Blue Roof	27.8	479.0	47
(G3) Green/Blue Roof	28.7	493.0	48
(G4) Green/Blue Roof	0.9	41.0	4
(G5) Green/Blue Roof	1.2	44.0	4
Roof Storage Sub-Total			327
Cellular Below Ground Attenuation Tank	68.7	179.0	114
Permeable Paving/Surfacing	4.3	120.0	18
Underground Storage Sub-Total			132
Total	270		459

5.4.3 SuDS

The document 'Sustainable Urban Drainage Systems' (SuDS) published by CIRIA, document No. C521, was utilised for the surface water design strategy for the proposed development. The document encourages the use of a variety of alternative measures in the design of sustainable drainage systems, which take account of quality, quantity and amenity. These measures protect or enhance water quality, are sympathetic to the environment, provide a habitat for wildlife and encourage natural ground water recharge. They provide storage that not only attenuates the flow but also permits settlement of coarse silts. Runoff would also be treated by absorption of particles by aquatic vegetation or by soil, and by biological activity.

The SuDS features incorporated into the drainage design for the scheme include the following:

- Green/Blue Terrace;
- Green/Blue Roof;
- Permeable paving;
- Soft Landscaping
- SuDS Tree Pits

5.4.3.1 Green/Blue Roofs & Terraces

A typical extensive green roof build-up is included in Figure 5-4 below.

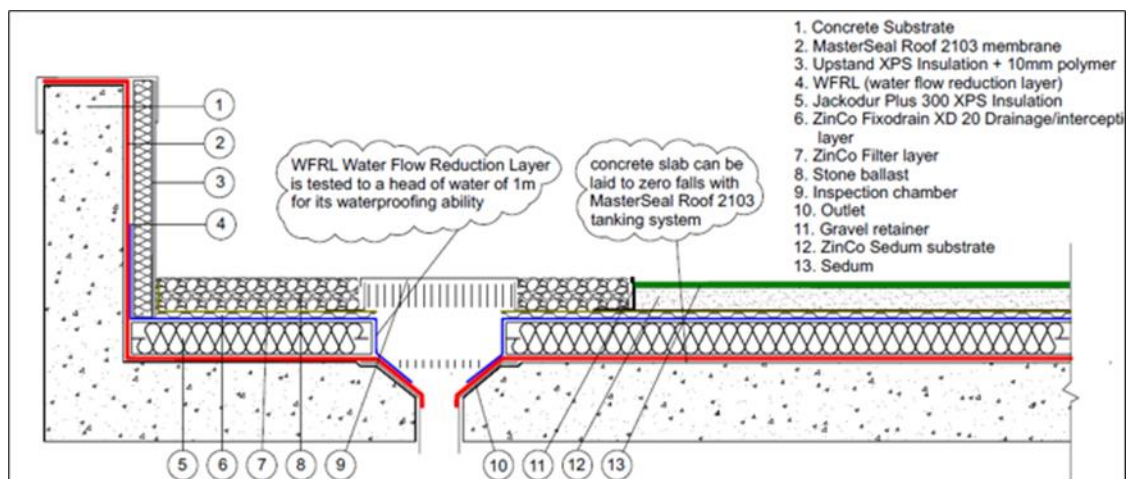


Figure 5-4 Typical Extensive Green Roof Detail

The proposed ABG D108 green/blue roof system build-up for the roofs and terraces is illustrated in Figure 5-5 below. The finishing no top of the blue roof storage layer would vary, depending on the final landscape layout and would be a mixture of soft and permeable hard surfaces, both contributing to surface water interception.

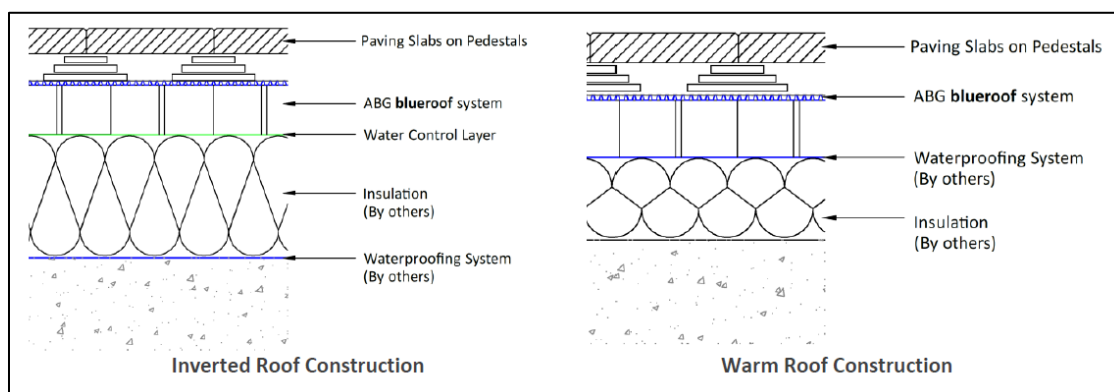


Figure 5-5 Blue Roof Detail - D108mm

The surface water drainage layout for the scheme is detailed in DBFL drawings no. 210178-DBFL-CS-SP-DR-C-1300 (Site Services) and 210178-DBFL-CS-SP-DR-C-1310 (Surface Water Catchment Plan).

5.4.3.2 Permeable Paving

The proposed design includes permeable paving finishes as indicated on drawings 210178-DBFL-CS-SP-DR-C-1310. Surface water runoff from the paved areas is intercepted by the permeable build-up of the paved areas where it is intended to naturally infiltrate into the ground. If the porous build-up of the paving and the in-situ material beneath becomes saturated, surface water would



drain to the surface water pipe network through gullies and linear drains as indicated on drawing 210178-DBFL-CS-SP-DR-C-1300.

Permeable paving reduces pollutants such as petrol and diesel as it contributes to its biodegrading process. It also assists in filtering solid particles out of surface water runoff, providing filtration before discharging into the surface water pipe network and ultimately the receiving watercourse.

5.4.3.3 SuDS Tree Pits

Tree Pits are proposed to intercept excess surface water runoff from soft landscaping areas between buildings within the development. SuDS tree pit details as proposed are shown on drawing 210178-DBFL-CS-SP-DR-C-5303. Tree pits are slightly lowered below surrounding surface area to allow surface water to be directly intercepted at the base of the tree pit and infiltrate down to the tree root system. Once the tree pit is saturated, water would be routed to the surface water pipe network through an overflow as detailed on drawing 210178-DBFL-CS-SP-DR-C-5303.

5.4.4 Surface Water Drainage Design Standards

Surface water drainage for the proposed development is designed using the recommendations of the GDS DS, EN752 and BS8301:1985, with the following parameters applied:

- Return period for pipe network 2 years,
 - check 30-year 15 minute, no flooding;
 - check 100-year flooding in designated areas;
- Time of entry 4 minutes
- Pipe Friction (Ks) (concrete) 0.6 mm
- Minimum Velocity 1.0 m/s
- Standard Average Annual Rainfall 729mm
- M5-60 16.1mm
- Ratio r (M5-60/M5-2D) 0.278
- Storage System Storm Return Event GDS DS Volume 2, p61, Criterion 3
 - 10-year no flooding on site;
 - 30-year no flooding on site;



- 100-year check no internal property flooding. Flood routing plan. FFL + 500mm freeboard above 100-year flood level. No flooding to adjacent areas.
- Climate Change 20% for rainfall intensities.
- C_v winter 0.84
- C_v summer 0.75

(Note on C_v Factors; The DOE "Recommendations for Site Development Works" & BS8301:1985 note that the volumetric runoff coefficient C_v should be set at 0.6 for rapidly draining soils and 0.9 for heavy soils. Applying a C_v rate of 0.84 for Winter and 0.75 for Summer is standard practice and is appropriate for this site.

The *Network Module* of *Microdrainage* has been used to assess the performance of the proposed surface water network, attenuation structures and controlled discharge into the existing public surface water network in Richmond Road. This analysis indicated that the pipe sizes and grades are adequate for storm events up to the 1% AEP and that the underground cellular storage volume of 114m³ cellular storage at ground level satisfies attenuation requirements along with the storage provided in the green/blue roofs and terraces.

Refer to **Appendix C** for *Microdrainage Network Module* calculations.

A breakdown of the impermeable areas contributing to the surface water drainage network is included in *Table 5-2* below;

Table 5-2 Impermeable Areas

Catchment 1 - 158A Richmond Road Development			
Surface Type	Gross Area (ha)	Runoff Coefficient	Total Impermeable
Green/Blue Roof	0.141	0.9	0.127
Green/Blue Podium	0.141	0.9	0.126
Footpaths on Green/Blue Podium	0.063	0.9	0.057
Roof Area (Traditional)	0.056	1.0	0.056
Permeable Paving	0.012	0.8	0.010
Soft Landscaped Areas	0.040	0.3	0.012
Hard Landscaping Areas	0.083	1.0	0.083
Total	0.536		0.471

The green/blue roof area and the green/blue terrace areas make up a total of **70%** of the total roof/terrace area as calculated in *Table 5-3* below $(0.141+0.141)/(0.40)*100 = 70\%$, satisfying the minimum 70% required by DCC for extensive green/blue roofs stipulated in the Green & Blue Roof Guide, 2021 and section 15.6.3 of the DCC Development Plan 2022 – 2028..



Table 5-3 Roof/Terrace Areas

Roof Area Breakdown	
Surface Type	Gross Area (ha)
Green/Blue Roof	0.141
Green/Blue Terrace	0.141
Footpaths on Green/Blue Terrace	0.063
Roof Area (Traditional)	0.056
Total	0.40

5.4.5 Interception and Treatment Storage

To prevent pollutants or sediments discharging into water courses the GDSDS requires “interception storage” to be incorporated into the development.

The volume of interception required is based on 5mm of rainfall depth from 80% of the runoff from impermeable areas as defined in GDSDS. The interception volume attributable to each SuDS feature consists of the volume of water that can infiltrate to the ground, evaporate into the atmosphere or can transpire through plants and vegetation. Additionally, there will be some losses of water due to absorption, wetting of stone and soil media.

Interception is provided as follows:

- Green/blue roof runoff is intercepted in the green roof build-up.
- Green/Blue terrace runoff is intercepted in the build-up of the green areas on the terrace and stone build-up of the hard terrace areas.
- Footpath runoff in landscaped areas at ground level are intercepted by the specified permeable paving or adjacent soft landscaping where impermeable paving is used.

All surfaced water collected from roads will pass through an appropriate petrol interceptor and grease trap, complying with the provision of 2 treatment stages mentioned within requirements of the CIRIA document C697.

5.5 Surface Water Management – Richmond Road Works

Surface water run-off from Richmond Road currently drains along the road in a south easterly direction as shown in Figure 5-6, where it drains into existing gullies connected to the existing 1350mm surface water sewer at the south eastern corner of the proposed development.



Figure 5-6 Richmond Road Works Surface Water Management

A new surface water sewer is proposed for the section of Richmond Road to be upgraded as indicated on drawing 210178-DBFL-CS-SP-DR-C-1300. The proposed 300mm surface water sewer would drain runoff from the road, footpaths and cycle tracks/lanes through road gullies and tie in at the same proposed manhole where the proposed development's surface water is proposed to discharge. All rainfall parameters for the Richmond Road works match the parameters for the internal surface water sewer network as listed in section 4.4.4.

Surface water network calculations for the Richmond Road surface water sewer network generated with the Micro-Drainage Network module are included in **Appendix C**.

5.6 Flood Risk

A separate '*Site Specific flood Risk Assessment*' (SSFRA) by DBFL Consulting Engineers, is included under separate cover, and addresses the flood protection measures for the proposed development.



5.7 DCC Surface Water Management Policies

The Surface Water Management Plan as detailed in this chapter satisfied the relevant surface water management policies as stipulated in *Chapter 9: Sustainable Environmental Infrastructure and Flood Risk* of the DCC Development Plan 2022 – 2028.

The relevant DCC surface water management policies are listed below:

SI7 Water Quality Status – Water quality enhanced by nature based drainage features

SI8 Physical Condition of Waterbodies - N/A

SI9 Groundwater Pollution – Groundwater pollution reduced through SuDS features

SI10 Managing Development Within and Adjacent to River Corridors – N/A

SI11 Managing Development Within and Adjacent to Camac River Corridor – N/A

SI12 River Restoration in Strategic Development and Regeneration Areas - N/A

SI13 Minimising Flood Risk – Addressed in the SSFRA

SI14 Strategic Flood Risk Assessment – Addressed in the SSFRA

SI15 Site-Specific Flood Risk Assessment – SSFRA as per the DCC 2022-2028 SFRA enclosed

SI16 Site-Specific Flood Risk Assessment– N/A

SI17 Catchment-Based Flood Risk Management Plans – Catchment plan drawing enclosed dr 210178-DBFL-SW-SP-DR-C-1310

SI18 Protection of Flood Alleviation Infrastructure

SI19 Provision and Upgrading of Flood Alleviation Assets – Flood wall provided as per dr 210178-DBFL-RD-SP-DR-C-5211

SI20 Basement Flood Risk Management – No Basement/ N/A

SI21 Managing Surface Water Flood Risk - SUDS and nature based solutions included as per section 5.4.3 of the Infrastructure Design Report

SI22 Sustainable Drainage Systems – included in design as per section 5.4.3 of the Infrastructure Design Report

SI23 Green Blue Roofs – Included in design as per section 5.4.3 of the Infrastructure Design Report

SI24 Control of Paving of Private Driveways / Vehicular Entrances / Grassed Areas



SI25 Surface Water Management – Included as part of this report

SI26 Taking in Charge of Private Drainage Infrastructure – Public Road drainage and connections will be to DCC standard. The Development will be private building drainage – it is assumed that building drainage does not need to be constructed to DCC standard (e.g. Ajs, Inspection chambers, attenuation storage etc)

6 FOUL DRAINAGE

There is an existing 900mm concrete foul sewer in Richmond Road. It is proposed to discharge foul flows from the development to the existing 900mm located on Richmond Road at the proposed site entrance at the existing manhole indicated in *Figure 6-1* below.

A pre-connection enquiry form for the subject site was issued to Irish Water and a copy of the Confirmation of Feasibility (COF) from Irish Water is included in **Appendix E**.

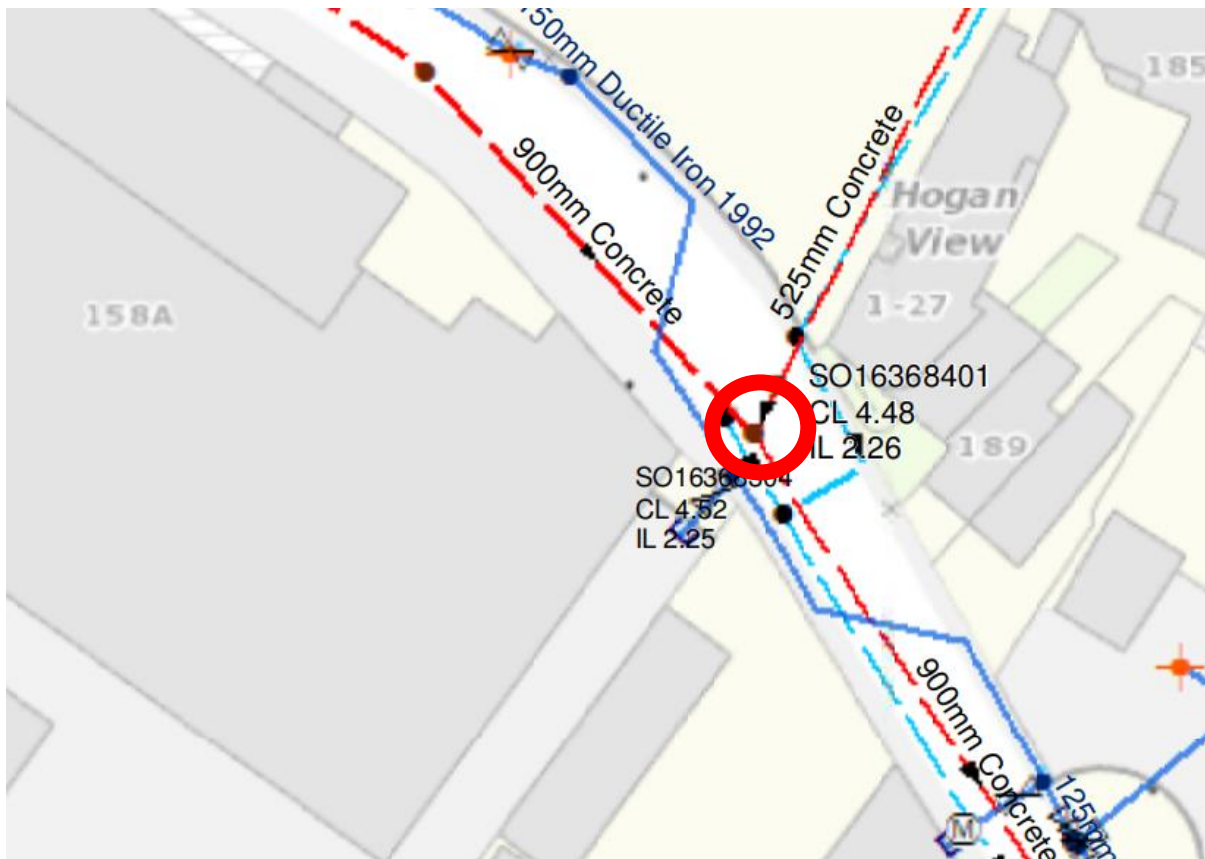


Figure 6-1 Extract of Irish Water Records (Foul connection)

In accordance with the conditions set by Irish Water within the confirmation of feasibility, foul and surface water flows from the development will be conveyed in separate networks.

The design of the foul water network was issued to Irish Water and the letter with the Statement of Design Acceptance from Irish Water is included in **Appendix F**.

Foul sewers have been designed in accordance with the Building Regulations and specifically in accordance with the principles and methods set out in the DOE "Recommendations for Site



Development Works for Housing Areas", IS EN752 (2008), BS8301: 1985, IS EN12056: Part 2 (2000) and the recommendations of the 'Greater Dublin Strategic Drainage Study', (GDSDS).

The following criteria have been applied:

Demand	446l/house/day (Irish Water Code of Practice)
Discharge units	14 units per house (BS8301:1985)
Pipe Friction (Ks)	1.5 mm (concrete)
Pipe Friction (Ks)	0.15 mm (uPVC)
Minimum Velocity	0.75 m/s (self-cleansing velocity)
Maximum Velocity	3.0 m/s (1:20 maximum pipe gradient)
Frequency Factor	0.5 for domestic use

Estimated foul loading generated by the development are included in Table 6-1 below:

Table 6-1 Foul Loading Demand

TITLE

158A Richmond Road

Job Reference

210178

SUBJECT

Wastewater Demand for Irish Water

Calc. Sheet No.

1

DRAWING NUMBER

-

Calculations by

DB

Checked by

KS

Date

24/01/2023



RESIDENTIAL - PREDICTED DEVELOPMENT FOUL FLOWS

Unit Type	No.	Loading l/person/day	Occupancy person/unit	Occupancy	Daily Loading l/day	Daily Loading l/s
2 Bed	68	150	2.7	184	27,540	0.32
1 Bed	65	150	2.7	176	26,325	0.30
Residential Daily Loading						0.62
Growth Factor						1
Infiltration @ 10% (as CoP App C 1.2.4)						0.06
Dry Weather Flow l/s						0.69
Residential Peak Factor (as CoP App C 1.2.5)						6.0
Design Foul Flow l/s						4.11

*Flow rates calculated using IW CoP for Wastewater Infrastructure Appendix C

NON-RESIDENTIAL - PREDICTED DEVELOPMENT FOUL FLOWS

Unit Type	Floor Area m ²	Occupancy Load m ² /person	Occupancy	Loading l/Person/day	Daily Loading l/day	Daily Loading l/s
Commercial (Retail)	1,111	50	22	12	267	0.00
Community Area (Artist Studio)	749	30	25	50	1,248	0.01
Non - Residential Daily Loading						0.02
Growth Factor						1
Infiltration @ 10% (as CoP App C 1.2.4)						0.00
Dry Weather Flow l/s						0.02
Commercial Peak Factor (as CoP App C 1.2.7)						6
Design Foul Flow l/s						0.12

TOTAL PREDICTED DEVELOPMENT AVERAGE FOUL FLOWS l/s

0.64

TOTAL PREDICTED DEVELOPMENT PEAK FOUL FLOWS l/s

4.23

Refer to **Appendix D** for the Micro Drainage Foul Drainage Network calculations.

7 WATER SUPPLY AND DISTRIBUTION

The water main distribution system for the development will connect to the existing 150mm diameter water main located in Richmond Road as shown in Figure 7-1. A pre-connection enquiry form for the subject site was issued to Irish Water and a copy of the Confirmation of Feasibility (COF) from Irish Water is included in **Appendix E**.

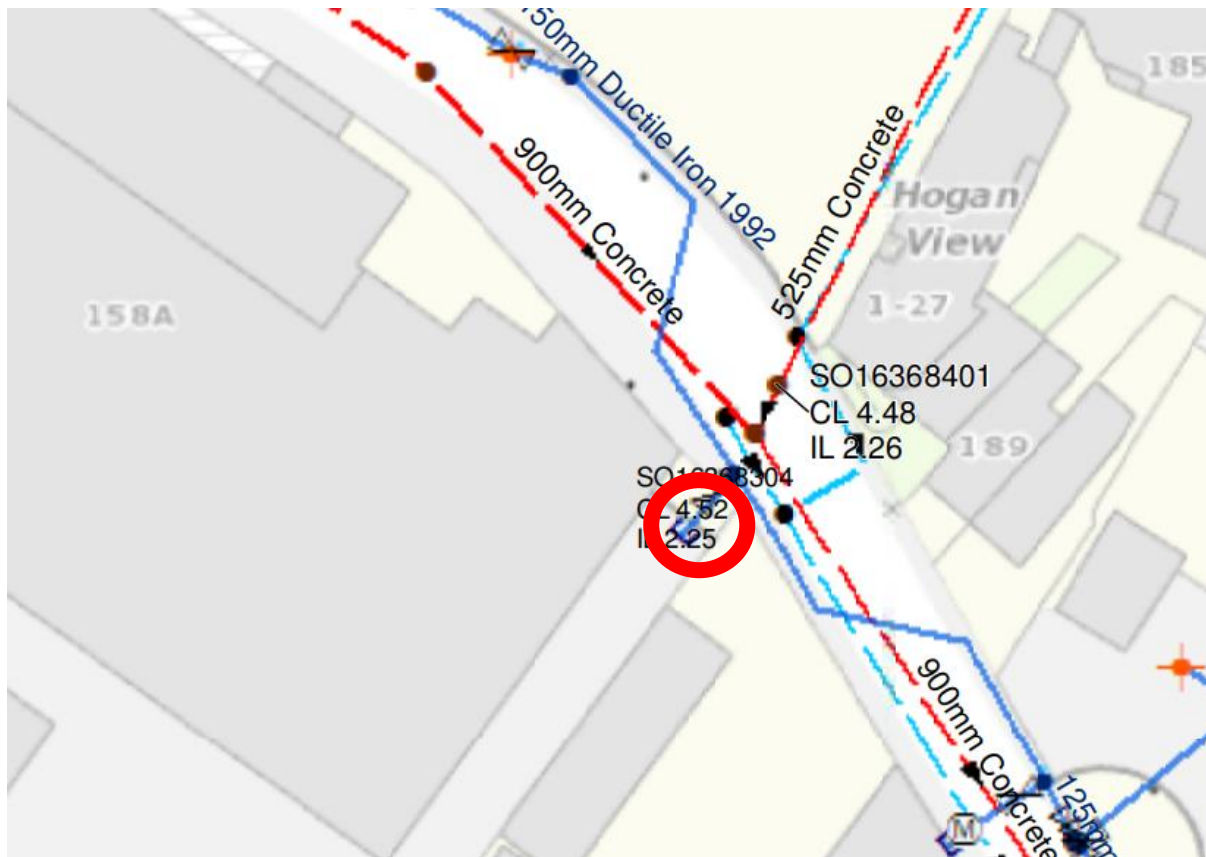


Figure 7-1 Extract of Irish Water Records (Water connection)

Water for firefighting purposes will be routed from the tank room to a separate sprinkler tank room on the ground floor of the site, from where water will be distributed internally to the sprinkler system. 2 Hydrants are located in Richmond Road within 46m of the proposed buildings for firefighting purposes.

The estimated water daily water demand generated by the development is included in Table 7-1 below.



Table 7-1 Water Demand

TITLE		Job Reference		
158A Richmond Road		210178		
SUBJECT		Calc. Sheet No.		
Water Demand for Irish Water		2		
DRAWING NUMBER	Calculations by	Checked by	Date	
-	DB	KS	24/01/2023	

RESIDENTIAL - WATER DEMAND						
Unit Type	No. Dwellings	Occupancy Rate /dwelling	Occupancy	Per Capita Consumption l/Person/day	Average Daily Domestic Demand l/day	Average Daily Domestic Demand l/s
2 Bed	68	2.7	184	150	27,540	0.32
1 Bed	65	2.7	176	150	26,325	0.30
Total Average Daily Loading l/s						0.62
Average Day/Week Domestic Demand						1.25
Average Day/Peak Week Demand l/s						0.78
Peak Demand Factor						5
Peak Hour Water Demand l/s						3.90
*Flow rates calculated using IW CoP for Water Infrastructure						

NON-RESIDENTIAL WATER DEMAND						
Unit Type	Floor Area m ²	Occupancy Rate m ² /person	Occupancy	Per Capita Consumption l/Person/day	Average Daily Demand l/day	Average Daily Demand l/s
Commercial (Retail)	1,111	50	22	150	3,333	0.04
Community Area (Artist Studio)	749	30	25	150	3,745	0.04
Total Average Daily Loading l/s						0.08
Average Day/Week Demand						1.25
Average Day/Peak Week Demand l/s						0.10
Peak Demand Factor						5
Peak Hour Water Demand l/s						0.51
*Flow rates calculated using IW CoP for Water Infrastructure						

TOTAL AVERAGE DAILY LOADING l/s						0.71
AVERAGE DAY/PEAK WEEK DEMAND l/s						0.88
PEAK HOUR WATER DEMAND						4.41
*Flow rates calculated using IW CoP for Wastewater Infrastructure Appendix D						

The design of the watermain was issued to Irish Water and the letter with the Statement of Design Acceptance from Irish Water is included in **Appendix F**.



Appendix A : Met Eireann Rainfall Data

Met Eireann
Return Period Rainfall Depths for sliding Durations
Irish Grid: Easting: 316654, Northing: 236444,

DURATION	Interval		Years													
	6months,	1year,	2,	3,	4,	5,	10,	20,	30,	50,	75,	100,	150,	200,	250,	500,
5 mins	2.5,	3.6,	4.1,	5.0,	5.6,	6.0,	7.5,	9.2,	10.3,	11.8,	13.2,	14.3,	16.0,	17.3,	18.4,	N/A ,
10 mins	3.5,	5.0,	5.7,	6.9,	7.7,	8.4,	10.4,	12.8,	14.3,	16.5,	18.4,	20.0,	22.3,	24.1,	25.6,	N/A ,
15 mins	4.1,	5.8,	6.8,	8.2,	9.1,	9.8,	12.3,	15.0,	16.8,	19.4,	21.7,	23.5,	26.2,	28.4,	30.2,	N/A ,
30 mins	5.4,	7.6,	8.8,	10.5,	11.7,	12.6,	15.5,	18.9,	21.1,	24.2,	26.9,	29.0,	32.3,	34.8,	36.9,	N/A ,
1 hours	7.2,	9.9,	11.4,	13.5,	15.0,	16.1,	19.7,	23.8,	26.4,	30.1,	33.4,	35.9,	39.8,	42.7,	45.2,	N/A ,
2 hours	9.5,	12.9,	14.7,	17.4,	19.2,	20.6,	25.0,	29.9,	33.1,	37.5,	41.4,	44.4,	49.0,	52.5,	55.4,	N/A ,
3 hours	11.2,	15.1,	17.2,	20.2,	22.2,	23.7,	28.7,	34.2,	37.7,	42.6,	46.9,	50.2,	55.3,	59.2,	62.3,	N/A ,
4 hours	12.6,	16.9,	19.1,	22.4,	24.6,	26.3,	31.7,	37.6,	41.4,	46.7,	51.3,	54.9,	60.3,	64.4,	67.8,	N/A ,
6 hours	14.8,	19.7,	22.3,	26.0,	28.5,	30.3,	36.4,	43.0,	47.2,	53.1,	58.2,	62.1,	68.1,	72.6,	76.3,	N/A ,
9 hours	17.4,	23.0,	25.9,	30.1,	32.9,	35.0,	41.8,	49.1,	53.9,	60.4,	66.0,	70.3,	76.9,	81.8,	85.9,	N/A ,
12 hours	19.5,	25.7,	28.9,	33.5,	36.5,	38.8,	46.1,	54.0,	59.1,	66.1,	72.2,	76.8,	83.8,	89.1,	93.5,	N/A ,
18 hours	23.0,	30.0,	33.6,	38.6,	42.2,	44.8,	53.0,	61.8,	67.5,	75.2,	81.9,	87.0,	94.6,	100.5,	105.2,	N/A ,
24 hours	25.8,	33.5,	37.4,	43.1,	46.8,	49.6,	58.4,	68.0,	74.1,	82.3,	89.5,	95.0,	103.2,	109.4,	114.5,	131.8,
2 days	31.6,	40.2,	44.6,	50.8,	54.9,	57.9,	67.5,	77.7,	84.1,	92.9,	100.4,	106.0,	114.5,	120.9,	126.2,	143.8,
3 days	36.2,	45.7,	50.4,	57.1,	61.4,	64.7,	74.8,	85.6,	92.4,	101.6,	109.4,	115.3,	124.1,	130.8,	136.2,	154.4,
4 days	40.2,	50.4,	55.4,	62.5,	67.1,	70.5,	81.3,	92.6,	99.7,	109.2,	117.4,	123.5,	132.6,	139.5,	145.0,	163.7,
6 days	47.3,	58.6,	64.1,	72.0,	77.0,	80.7,	92.4,	104.6,	112.2,	122.5,	131.2,	137.7,	147.3,	154.6,	160.5,	180.1,
8 days	53.5,	65.8,	71.8,	80.2,	85.6,	89.6,	102.1,	115.1,	123.2,	134.0,	143.2,	150.0,	160.2,	167.8,	173.9,	194.5,
10 days	59.1,	72.3,	78.7,	87.7,	93.4,	97.7,	110.9,	124.6,	133.1,	144.4,	154.0,	161.1,	171.8,	179.7,	186.1,	207.4,
12 days	64.4,	78.4,	85.2,	94.6,	100.7,	105.2,	119.0,	133.3,	142.2,	154.0,	164.0,	171.4,	182.4,	190.6,	197.3,	219.3,
16 days	74.2,	89.5,	97.0,	107.3,	113.9,	118.8,	133.8,	149.3,	158.8,	171.5,	182.2,	190.1,	201.8,	210.5,	217.6,	240.9,
20 days	83.1,	99.8,	107.8,	119.0,	126.0,	131.3,	147.3,	163.8,	173.9,	187.3,	198.6,	207.0,	219.4,	228.5,	235.9,	260.4,
25 days	93.6,	111.7,	120.4,	132.4,	140.0,	145.7,	162.8,	180.4,	191.2,	205.5,	217.5,	226.4,	239.5,	249.2,	257.0,	282.7,

NOTES:

N/A Data not available

These values are derived from a Depth Duration Frequency (DDF) Model

For details refer to:

'Fitzgerald D. L. (2007), Estimates of Point Rainfall Frequencies, Technical Note No. 61, Met Eireann, Dublin',
Available for download at www.met.ie/climate/dataproducts/Estimation-of-Point-Rainfall-Frequencies_TN61.pdf



Appendix B : Surface Water Storage Calculations [Micro Drainage Cascade Source Control]

DBFL Consulting Engineers				Page 1	
Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 20:55		Designed by besterd			
File 20220829 Cascade.CASX		Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 Cellular Tank.SRCX</u>					
Upstream Structures		Outflow To Overflow To			
220830 GreenBlue Podium (P3).SRCX		(None)		(None)	
220830 GreenBlue Podium (P4).SRCX					
220830 GreenBlue Podium (P5).SRCX					
220830 GreenBlue Podium (P6).SRCX					
220830 GreenBlue Podium (P7).SRCX					
220830 GreenBlue Roof (G1).SRCX					
220830 GreenBlue Roof (G2).SRCX					
220830 GreenBlue Roof (G3).SRCX					
230106 PermPaving.SRCX					
220830 GreenBlue Podium (P8).SRCX					
220830 GreenBlue Roof (G4).SRCX					
220830 GreenBlue Roof (G5).SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	3.272	0.100	1.9	18.1	O K
30 min Summer	3.309	0.137	1.9	24.6	O K
60 min Summer	3.346	0.174	2.0	31.3	O K
120 min Summer	3.385	0.213	2.0	38.3	O K
180 min Summer	3.408	0.236	2.0	42.5	O K
240 min Summer	3.425	0.253	2.0	45.5	O K
360 min Summer	3.448	0.276	2.0	49.6	O K
480 min Summer	3.463	0.291	2.0	52.4	O K
600 min Summer	3.474	0.302	2.0	54.4	O K
720 min Summer	3.482	0.310	2.0	55.7	O K
960 min Summer	3.489	0.317	2.0	57.0	O K
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	39.2	18	
30 min Summer	58.468	0.0	57.6	33	
60 min Summer	38.020	0.0	94.2	62	
120 min Summer	23.997	0.0	121.7	122	
180 min Summer	18.178	0.0	139.4	182	
240 min Summer	14.896	0.0	152.8	242	
360 min Summer	11.219	0.0	172.8	362	
480 min Summer	9.163	0.0	187.8	482	
600 min Summer	7.826	0.0	199.6	602	
720 min Summer	6.878	0.0	209.2	722	
960 min Summer	5.608	0.0	224.1	960	
©1982-2020 Innovyze					

DBFL Consulting Engineers					Page 2	
Ormond House Upper Ormond Quay Dublin 7						
Date 23/02/2023 20:55 File 20220829 Cascade.CASX		Designed by besterd Checked by				
Innovyze			Source Control 2020.1			
<u>Cascade Summary of Results for 220830 Cellular Tank.SRCX</u>						
Storm Event		Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
1440 min Summer		3.490	0.318	2.0	57.2	O K
2160 min Summer		3.484	0.312	2.0	56.1	O K
2880 min Summer		3.473	0.301	2.0	54.1	O K
4320 min Summer		3.441	0.269	2.0	48.3	O K
5760 min Summer		3.404	0.232	2.0	41.8	O K
7200 min Summer		3.370	0.198	2.0	35.6	O K
8640 min Summer		3.339	0.167	2.0	30.1	O K
10080 min Summer		3.314	0.142	2.0	25.5	O K
15 min Winter		3.285	0.113	1.9	20.3	O K
30 min Winter		3.327	0.155	2.0	27.8	O K
60 min Winter		3.370	0.198	2.0	35.6	O K
120 min Winter		3.415	0.243	2.0	43.8	O K
180 min Winter		3.443	0.271	2.0	48.9	O K
240 min Winter		3.464	0.292	2.0	52.6	O K
360 min Winter		3.494	0.322	2.0	57.9	O K
480 min Winter		3.514	0.342	2.0	61.6	O K
600 min Winter		3.528	0.356	2.0	64.1	O K
720 min Winter		3.538	0.366	2.0	65.9	O K
960 min Winter		3.550	0.378	2.0	68.1	O K
Storm Event		Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
1440 min Summer		4.204	0.0	242.5	1224	
2160 min Summer		3.150	0.0	323.6	1600	
2880 min Summer		2.565	0.0	344.9	1988	
4320 min Summer		1.917	0.0	368.5	2764	
5760 min Summer		1.558	0.0	458.0	3520	
7200 min Summer		1.326	0.0	483.2	4256	
8640 min Summer		1.162	0.0	501.3	4960	
10080 min Summer		1.039	0.0	512.7	5648	
15 min Winter		84.523	0.0	44.8	18	
30 min Winter		58.468	0.0	65.8	33	
60 min Winter		38.020	0.0	106.9	62	
120 min Winter		23.997	0.0	137.6	122	
180 min Winter		18.178	0.0	157.3	182	
240 min Winter		14.896	0.0	172.2	240	
360 min Winter		11.219	0.0	194.3	360	
480 min Winter		9.163	0.0	210.4	478	
600 min Winter		7.826	0.0	223.0	596	
720 min Winter		6.878	0.0	233.2	712	
960 min Winter		5.608	0.0	248.6	942	
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 20:55 File 20220829 Cascade.CASX		Designed by besterd Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 Cellular Tank.SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
1440 min Winter	3.554	0.382	2.0	68.7	O K
2160 min Winter	3.535	0.363	2.0	65.4	O K
2880 min Winter	3.513	0.341	2.0	61.5	O K
4320 min Winter	3.455	0.283	2.0	50.9	O K
5760 min Winter	3.393	0.221	2.0	39.7	O K
7200 min Winter	3.341	0.169	2.0	30.5	O K
8640 min Winter	3.302	0.130	1.9	23.4	O K
10080 min Winter	3.274	0.102	1.9	18.4	O K
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
1440 min Winter	4.204	0.0	266.9	1380	
2160 min Winter	3.150	0.0	359.6	1752	
2880 min Winter	2.565	0.0	382.0	2164	
4320 min Winter	1.917	0.0	408.0	2984	
5760 min Winter	1.558	0.0	512.0	3744	
7200 min Winter	1.326	0.0	538.6	4400	
8640 min Winter	1.162	0.0	557.6	5104	
10080 min Winter	1.039	0.0	569.6	5752	
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Ormond House Upper Ormond Quay Dublin 7		
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Innovyze		Source Control 2020.1

Cascade Rainfall Details for 220830 Cellular Tank.SRCX

Rainfall Model	FSR	Winter Storms	Yes
Return Period (years)	100	Cv (Summer)	0.750
Region	Scotland and Ireland	Cv (Winter)	0.840
M5-60 (mm)	16.100	Shortest Storm (mins)	15
Ratio R	0.278	Longest Storm (mins)	10080
Summer Storms	Yes	Climate Change %	+20

Time Area Diagram

Total Area (ha) 0.120

Time (mins)	Area
From: To:	(ha)
0	4 0.120

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<u>Cascade Model Details for 220830 Cellular Tank.SRCX</u> Storage is Online Cover Level (m) 4.100 <u>Tank or Pond Structure</u> Invert Level (m) 3.172 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>180.0</td><td>0.660</td><td>180.0</td><td>0.661</td><td>0.0</td></tr></table> <u>Hydro-Brake® Optimum Outflow Control</u> <table><tr><td>Unit Reference</td><td>MD-SHE-0070-2000-0776-2000</td></tr><tr><td>Design Head (m)</td><td>0.776</td></tr><tr><td>Design Flow (l/s)</td><td>2.0</td></tr><tr><td>Flush-Flo™</td><td>Calculated</td></tr><tr><td>Objective</td><td>Minimise upstream storage</td></tr><tr><td>Application</td><td>Surface</td></tr><tr><td>Sump Available</td><td>Yes</td></tr><tr><td>Diameter (mm)</td><td>70</td></tr><tr><td>Invert Level (m)</td><td>3.156</td></tr><tr><td>Minimum Outlet Pipe Diameter (mm)</td><td>100</td></tr><tr><td>Suggested Manhole Diameter (mm)</td><td>1200</td></tr></table> <table><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr><tr><td>Design Point (Calculated)</td><td>0.776</td><td>2.0</td><td>Kick-Flo®</td><td>0.493</td><td>1.6</td></tr><tr><td>Flush-Flo™</td><td>0.232</td><td>2.0</td><td>Mean Flow over Head Range</td><td>-</td><td>1.7</td></tr></table> The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated <table><tr><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th></tr><tr><td>0.100</td><td>1.8</td><td>1.200</td><td>2.4</td><td>3.000</td><td>3.7</td><td>7.000</td><td>5.5</td></tr><tr><td>0.200</td><td>2.0</td><td>1.400</td><td>2.6</td><td>3.500</td><td>4.0</td><td>7.500</td><td>5.7</td></tr><tr><td>0.300</td><td>2.0</td><td>1.600</td><td>2.8</td><td>4.000</td><td>4.3</td><td>8.000</td><td>5.9</td></tr><tr><td>0.400</td><td>1.9</td><td>1.800</td><td>2.9</td><td>4.500</td><td>4.5</td><td>8.500</td><td>6.1</td></tr><tr><td>0.500</td><td>1.6</td><td>2.000</td><td>3.1</td><td>5.000</td><td>4.7</td><td>9.000</td><td>6.3</td></tr><tr><td>0.600</td><td>1.8</td><td>2.200</td><td>3.2</td><td>5.500</td><td>4.9</td><td>9.500</td><td>6.4</td></tr><tr><td>0.800</td><td>2.0</td><td>2.400</td><td>3.4</td><td>6.000</td><td>5.1</td><td></td><td></td></tr><tr><td>1.000</td><td>2.2</td><td>2.600</td><td>3.5</td><td>6.500</td><td>5.3</td><td></td><td></td></tr></table>						Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	0.000	180.0	0.660	180.0	0.661	0.0	Unit Reference	MD-SHE-0070-2000-0776-2000	Design Head (m)	0.776	Design Flow (l/s)	2.0	Flush-Flo™	Calculated	Objective	Minimise upstream storage	Application	Surface	Sump Available	Yes	Diameter (mm)	70	Invert Level (m)	3.156	Minimum Outlet Pipe Diameter (mm)	100	Suggested Manhole Diameter (mm)	1200	Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	0.776	2.0	Kick-Flo®	0.493	1.6	Flush-Flo™	0.232	2.0	Mean Flow over Head Range	-	1.7	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	1.8	1.200	2.4	3.000	3.7	7.000	5.5	0.200	2.0	1.400	2.6	3.500	4.0	7.500	5.7	0.300	2.0	1.600	2.8	4.000	4.3	8.000	5.9	0.400	1.9	1.800	2.9	4.500	4.5	8.500	6.1	0.500	1.6	2.000	3.1	5.000	4.7	9.000	6.3	0.600	1.8	2.200	3.2	5.500	4.9	9.500	6.4	0.800	2.0	2.400	3.4	6.000	5.1			1.000	2.2	2.600	3.5	6.500	5.3		
Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)																																																																																																																												
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 20:56			Designed by besterd		
File 20220829 Cascade.CASX			Checked by		
Innovyze			Source Control 2020.1		
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P3).SRCX</u>					
Upstream Structures		Outflow To		Overflow To	
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	9.664	0.014	0.0	3.5	Flood Risk
30 min Summer	9.669	0.019	0.1	4.8	Flood Risk
60 min Summer	9.675	0.025	0.1	6.1	Flood Risk
120 min Summer	9.680	0.030	0.1	7.4	Flood Risk
180 min Summer	9.683	0.033	0.2	8.2	Flood Risk
240 min Summer	9.685	0.035	0.2	8.7	Flood Risk
360 min Summer	9.687	0.037	0.2	9.2	Flood Risk
480 min Summer	9.688	0.038	0.2	9.5	Flood Risk
600 min Summer	9.689	0.039	0.2	9.8	Flood Risk
720 min Summer	9.690	0.040	0.2	10.0	Flood Risk
960 min Summer	9.691	0.041	0.2	10.2	Flood Risk
1440 min Summer	9.692	0.042	0.2	10.4	Flood Risk
2160 min Summer	9.692	0.042	0.2	10.4	Flood Risk
2880 min Summer	9.691	0.041	0.2	10.1	Flood Risk
4320 min Summer	9.688	0.038	0.2	9.4	Flood Risk
5760 min Summer	9.685	0.035	0.2	8.7	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	2.0	19	
30 min Summer	58.468	0.0	3.0	34	
60 min Summer	38.020	0.0	5.1	64	
120 min Summer	23.997	0.0	6.7	122	
180 min Summer	18.178	0.0	7.7	182	
240 min Summer	14.896	0.0	8.5	242	
360 min Summer	11.219	0.0	9.7	354	
480 min Summer	9.163	0.0	10.6	406	
600 min Summer	7.826	0.0	11.3	464	
720 min Summer	6.878	0.0	11.9	526	
960 min Summer	5.608	0.0	13.0	662	
1440 min Summer	4.204	0.0	14.4	936	
2160 min Summer	3.150	0.0	17.8	1340	
2880 min Summer	2.565	0.0	19.3	1732	
4320 min Summer	1.917	0.0	21.2	2508	
5760 min Summer	1.558	0.0	24.2	3280	
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Ormond House Upper Ormond Quay Dublin 7					
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Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P3).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Summer	9.683	0.033	0.2	8.1	Flood Risk
8640 min Summer	9.681	0.031	0.1	7.6	Flood Risk
10080 min Summer	9.679	0.029	0.1	7.2	Flood Risk
15 min Winter	9.666	0.016	0.1	3.9	Flood Risk
30 min Winter	9.671	0.021	0.1	5.3	Flood Risk
60 min Winter	9.677	0.027	0.1	6.8	Flood Risk
120 min Winter	9.684	0.034	0.2	8.3	Flood Risk
180 min Winter	9.687	0.037	0.2	9.2	Flood Risk
240 min Winter	9.689	0.039	0.2	9.8	Flood Risk
360 min Winter	9.692	0.042	0.2	10.4	Flood Risk
480 min Winter	9.693	0.043	0.2	10.7	Flood Risk
600 min Winter	9.694	0.044	0.2	11.0	Flood Risk
720 min Winter	9.695	0.045	0.2	11.1	Flood Risk
960 min Winter	9.696	0.046	0.2	11.3	Flood Risk
1440 min Winter	9.696	0.046	0.2	11.3	Flood Risk
2160 min Winter	9.694	0.044	0.2	10.9	Flood Risk
2880 min Winter	9.692	0.042	0.2	10.3	Flood Risk
4320 min Winter	9.687	0.037	0.2	9.2	Flood Risk
5760 min Winter	9.683	0.033	0.2	8.2	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Summer	1.326	0.0	25.6	3968	
8640 min Summer	1.162	0.0	26.8	4752	
10080 min Summer	1.039	0.0	27.7	5448	
15 min Winter	84.523	0.0	2.3	19	
30 min Winter	58.468	0.0	3.5	33	
60 min Winter	38.020	0.0	5.9	62	
120 min Winter	23.997	0.0	7.6	120	
180 min Winter	18.178	0.0	8.7	178	
240 min Winter	14.896	0.0	9.6	234	
360 min Winter	11.219	0.0	11.0	346	
480 min Winter	9.163	0.0	12.0	448	
600 min Winter	7.826	0.0	12.8	480	
720 min Winter	6.878	0.0	13.5	556	
960 min Winter	5.608	0.0	14.6	710	
1440 min Winter	4.204	0.0	16.2	1010	
2160 min Winter	3.150	0.0	20.0	1444	
2880 min Winter	2.565	0.0	21.7	1848	
4320 min Winter	1.917	0.0	23.9	2636	
5760 min Winter	1.558	0.0	27.1	3400	
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File 20220829 Cascade.CASX		Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P3).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Winter	9.680	0.030	0.1	7.5	Flood Risk
8640 min Winter	9.678	0.028	0.1	6.8	Flood Risk
10080 min Winter	9.676	0.026	0.1	6.4	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Winter	1.326	0.0	28.8	4112	
8640 min Winter	1.162	0.0	30.1	4848	
10080 min Winter	1.039	0.0	31.1	5552	
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Application Surface																																																																													
Sump Available Yes																																																																													
Diameter (mm) 28																																																																													
Invert Level (m) 9.650																																																																													
Minimum Outlet Pipe Diameter (mm) 75																																																																													
Suggested Manhole Diameter (mm) 1200																																																																													
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 20:57 File 20220829 Cascade.CASX		Designed by besterd Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P4).SRCX</u>					
Upstream Structures		Outflow To		Overflow To	
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	23.764	0.014	0.0	2.0	Flood Risk
30 min Summer	23.769	0.019	0.1	2.8	Flood Risk
60 min Summer	23.774	0.024	0.1	3.5	Flood Risk
120 min Summer	23.779	0.029	0.1	4.2	Flood Risk
180 min Summer	23.781	0.031	0.1	4.5	Flood Risk
240 min Summer	23.782	0.032	0.2	4.7	Flood Risk
360 min Summer	23.784	0.034	0.2	5.0	Flood Risk
480 min Summer	23.785	0.035	0.2	5.1	Flood Risk
600 min Summer	23.785	0.035	0.2	5.2	Flood Risk
720 min Summer	23.786	0.036	0.2	5.3	Flood Risk
960 min Summer	23.786	0.036	0.2	5.3	Flood Risk
1440 min Summer	23.786	0.036	0.2	5.2	Flood Risk
2160 min Summer	23.784	0.034	0.2	5.0	Flood Risk
2880 min Summer	23.782	0.032	0.1	4.7	Flood Risk
4320 min Summer	23.778	0.028	0.1	4.2	Flood Risk
5760 min Summer	23.776	0.026	0.1	3.8	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	1.4	19	
30 min Summer	58.468	0.0	2.1	33	
60 min Summer	38.020	0.0	3.3	62	
120 min Summer	23.997	0.0	4.2	122	
180 min Summer	18.178	0.0	4.9	180	
240 min Summer	14.896	0.0	5.3	216	
360 min Summer	11.219	0.0	6.1	276	
480 min Summer	9.163	0.0	6.6	340	
600 min Summer	7.826	0.0	7.1	408	
720 min Summer	6.878	0.0	7.5	476	
960 min Summer	5.608	0.0	8.1	614	
1440 min Summer	4.204	0.0	9.1	880	
2160 min Summer	3.150	0.0	10.7	1276	
2880 min Summer	2.565	0.0	11.6	1644	
4320 min Summer	1.917	0.0	12.9	2380	
5760 min Summer	1.558	0.0	14.4	3112	
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10080 min Winter	1.039	0.0	18.6	5440	
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<u>Cascade Model Details for 220830 GreenBlue Podium (P4).SRCX</u> Storage is Online Cover Level (m) 23.950 <u>Tank or Pond Structure</u> Invert Level (m) 23.750 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>147.3</td><td>0.108</td><td>147.3</td><td>0.109</td><td>0.0</td></tr></table> <u>Hydro-Brake® Optimum Outflow Control</u> <table><tr><td>Unit Reference</td><td>MD-SHE-0028-2000-0109-2000</td></tr><tr><td>Design Head (m)</td><td>0.109</td></tr><tr><td>Design Flow (l/s)</td><td>0.2</td></tr><tr><td>Flush-Flo™</td><td>Calculated</td></tr><tr><td>Objective</td><td>Minimise upstream storage</td></tr><tr><td>Application</td><td>Surface</td></tr><tr><td>Sump Available</td><td>Yes</td></tr><tr><td>Diameter (mm)</td><td>28</td></tr><tr><td>Invert Level (m)</td><td>23.750</td></tr><tr><td>Minimum Outlet Pipe Diameter (mm)</td><td>75</td></tr><tr><td>Suggested Manhole Diameter (mm)</td><td>1200</td></tr></table> <table><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr><tr><td>Design Point (Calculated)</td><td>0.109</td><td>0.2</td><td>Kick-Flo®</td><td>0.079</td><td>0.2</td></tr><tr><td>Flush-Flo™</td><td>0.046</td><td>0.2</td><td>Mean Flow over Head Range</td><td>-</td><td>0.2</td></tr></table> The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated <table><tr><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th></tr><tr><td>0.100</td><td>0.2</td><td>1.200</td><td>0.6</td><td>3.000</td><td>0.9</td><td>7.000</td><td>1.3</td></tr><tr><td>0.200</td><td>0.3</td><td>1.400</td><td>0.6</td><td>3.500</td><td>1.0</td><td>7.500</td><td>1.4</td></tr><tr><td>0.300</td><td>0.3</td><td>1.600</td><td>0.6</td><td>4.000</td><td>1.0</td><td>8.000</td><td>1.4</td></tr><tr><td>0.400</td><td>0.3</td><td>1.800</td><td>0.7</td><td>4.500</td><td>1.1</td><td>8.500</td><td>1.5</td></tr><tr><td>0.500</td><td>0.4</td><td>2.000</td><td>0.7</td><td>5.000</td><td>1.1</td><td>9.000</td><td>1.5</td></tr><tr><td>0.600</td><td>0.4</td><td>2.200</td><td>0.8</td><td>5.500</td><td>1.2</td><td>9.500</td><td>1.6</td></tr><tr><td>0.800</td><td>0.5</td><td>2.400</td><td>0.8</td><td>6.000</td><td>1.2</td><td></td><td></td></tr><tr><td>1.000</td><td>0.5</td><td>2.600</td><td>0.8</td><td>6.500</td><td>1.3</td><td></td><td></td></tr></table>						Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	0.000	147.3	0.108	147.3	0.109	0.0	Unit Reference	MD-SHE-0028-2000-0109-2000	Design Head (m)	0.109	Design Flow (l/s)	0.2	Flush-Flo™	Calculated	Objective	Minimise upstream storage	Application	Surface	Sump Available	Yes	Diameter (mm)	28	Invert Level (m)	23.750	Minimum Outlet Pipe Diameter (mm)	75	Suggested Manhole Diameter (mm)	1200	Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	0.109	0.2	Kick-Flo®	0.079	0.2	Flush-Flo™	0.046	0.2	Mean Flow over Head Range	-	0.2	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	0.2	1.200	0.6	3.000	0.9	7.000	1.3	0.200	0.3	1.400	0.6	3.500	1.0	7.500	1.4	0.300	0.3	1.600	0.6	4.000	1.0	8.000	1.4	0.400	0.3	1.800	0.7	4.500	1.1	8.500	1.5	0.500	0.4	2.000	0.7	5.000	1.1	9.000	1.5	0.600	0.4	2.200	0.8	5.500	1.2	9.500	1.6	0.800	0.5	2.400	0.8	6.000	1.2			1.000	0.5	2.600	0.8	6.500	1.3		
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Ormond House Upper Ormond Quay Dublin 7						
Date 24/02/2023 15:35 File 20220829 Cascade.CASX		Designed by besterd Checked by				
Innovyze		Source Control 2020.1				
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P5).SRCX</u>						
Upstream Structures		Outflow To		Overflow To		
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX						
Storm Event		Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer		9.214	0.014	0.0	14.2	Flood Risk
30 min Summer		9.220	0.020	0.1	19.7	Flood Risk
60 min Summer		9.225	0.025	0.1	25.5	Flood Risk
120 min Summer		9.232	0.032	0.1	31.9	Flood Risk
180 min Summer		9.236	0.036	0.2	35.9	Flood Risk
240 min Summer		9.239	0.039	0.2	39.0	Flood Risk
360 min Summer		9.243	0.043	0.2	43.4	Flood Risk
480 min Summer		9.246	0.046	0.2	46.6	Flood Risk
600 min Summer		9.249	0.049	0.2	49.1	Flood Risk
720 min Summer		9.251	0.051	0.2	51.1	Flood Risk
960 min Summer		9.254	0.054	0.2	54.3	Flood Risk
1440 min Summer		9.258	0.058	0.2	58.6	Flood Risk
2160 min Summer		9.262	0.062	0.2	62.1	Flood Risk
2880 min Summer		9.264	0.064	0.2	64.0	Flood Risk
4320 min Summer		9.266	0.066	0.2	66.2	Flood Risk
5760 min Summer		9.267	0.067	0.2	67.1	Flood Risk
7200 min Summer		9.267	0.067	0.2	67.3	Flood Risk
8640 min Summer		9.267	0.067	0.2	66.9	Flood Risk
Storm Event		Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer		84.523	0.0	3.4	19	
30 min Summer		58.468	0.0	5.5	34	
60 min Summer		38.020	0.0	12.7	64	
120 min Summer		23.997	0.0	16.9	124	
180 min Summer		18.178	0.0	19.5	184	
240 min Summer		14.896	0.0	21.3	244	
360 min Summer		11.219	0.0	23.8	362	
480 min Summer		9.163	0.0	25.4	482	
600 min Summer		7.826	0.0	26.4	602	
720 min Summer		6.878	0.0	27.0	722	
960 min Summer		5.608	0.0	27.4	962	
1440 min Summer		4.204	0.0	26.2	1442	
2160 min Summer		3.150	0.0	51.5	2160	
2880 min Summer		2.565	0.0	51.2	2508	
4320 min Summer		1.917	0.0	46.2	3288	
5760 min Summer		1.558	0.0	86.6	4088	
7200 min Summer		1.326	0.0	89.1	4904	
8640 min Summer		1.162	0.0	89.1	5712	
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Ormond House Upper Ormond Quay Dublin 7					
Date 24/02/2023 15:35 File 20220829 Cascade.CASX		Designed by besterd Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P5).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
10080 min Summer	9.266	0.066	0.2	66.2	Flood Risk
15 min Winter	9.216	0.016	0.1	15.9	Flood Risk
30 min Winter	9.222	0.022	0.1	22.0	Flood Risk
60 min Winter	9.228	0.028	0.1	28.5	Flood Risk
120 min Winter	9.236	0.036	0.2	35.7	Flood Risk
180 min Winter	9.240	0.040	0.2	40.3	Flood Risk
240 min Winter	9.244	0.044	0.2	43.7	Flood Risk
360 min Winter	9.249	0.049	0.2	48.7	Flood Risk
480 min Winter	9.252	0.052	0.2	52.4	Flood Risk
600 min Winter	9.255	0.055	0.2	55.3	Flood Risk
720 min Winter	9.258	0.058	0.2	57.7	Flood Risk
960 min Winter	9.261	0.061	0.2	61.5	Flood Risk
1440 min Winter	9.266	0.066	0.2	66.6	Flood Risk
2160 min Winter	9.271	0.071	0.2	71.1	Flood Risk
2880 min Winter	9.273	0.073	0.2	73.4	Flood Risk
4320 min Winter	9.275	0.075	0.2	75.0	Flood Risk
5760 min Winter	9.275	0.075	0.2	75.5	Flood Risk
7200 min Winter	9.275	0.075	0.2	75.1	Flood Risk
8640 min Winter	9.274	0.074	0.2	73.9	Flood Risk
10080 min Winter	9.272	0.072	0.2	72.3	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
10080 min Summer	1.039	0.0	86.9	6560	
15 min Winter	84.523	0.0	4.0	19	
30 min Winter	58.468	0.0	6.5	34	
60 min Winter	38.020	0.0	14.7	64	
120 min Winter	23.997	0.0	19.3	122	
180 min Winter	18.178	0.0	22.0	182	
240 min Winter	14.896	0.0	24.0	240	
360 min Winter	11.219	0.0	26.5	358	
480 min Winter	9.163	0.0	27.8	476	
600 min Winter	7.826	0.0	28.5	594	
720 min Winter	6.878	0.0	28.8	710	
960 min Winter	5.608	0.0	28.5	944	
1440 min Winter	4.204	0.0	26.2	1402	
2160 min Winter	3.150	0.0	54.6	2080	
2880 min Winter	2.565	0.0	53.0	2740	
4320 min Winter	1.917	0.0	46.8	3628	
5760 min Winter	1.558	0.0	95.3	4440	
7200 min Winter	1.326	0.0	96.7	5400	
8640 min Winter	1.162	0.0	95.2	6304	
10080 min Winter	1.039	0.0	91.4	7168	
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<u>Cascade Summary of Results for 220830 GreenBlue Podium (P6).SRCX</u>					
Upstream Structures		Outflow To		Overflow To	
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	22.563	0.013	0.0	1.4	Flood Risk
30 min Summer	22.567	0.017	0.1	1.9	Flood Risk
60 min Summer	22.572	0.022	0.1	2.4	Flood Risk
120 min Summer	22.575	0.025	0.1	2.8	Flood Risk
180 min Summer	22.577	0.027	0.1	3.0	Flood Risk
240 min Summer	22.578	0.028	0.1	3.1	Flood Risk
360 min Summer	22.580	0.030	0.1	3.3	Flood Risk
480 min Summer	22.580	0.030	0.1	3.4	Flood Risk
600 min Summer	22.581	0.031	0.1	3.4	Flood Risk
720 min Summer	22.581	0.031	0.1	3.4	Flood Risk
960 min Summer	22.581	0.031	0.1	3.4	Flood Risk
1440 min Summer	22.579	0.029	0.1	3.3	Flood Risk
2160 min Summer	22.577	0.027	0.1	3.0	Flood Risk
2880 min Summer	22.576	0.026	0.1	2.8	Flood Risk
4320 min Summer	22.573	0.023	0.1	2.5	Flood Risk
5760 min Summer	22.571	0.021	0.1	2.3	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	1.0	19	
30 min Summer	58.468	0.0	1.6	33	
60 min Summer	38.020	0.0	2.3	62	
120 min Summer	23.997	0.0	3.0	120	
180 min Summer	18.178	0.0	3.4	162	
240 min Summer	14.896	0.0	3.8	190	
360 min Summer	11.219	0.0	4.3	254	
480 min Summer	9.163	0.0	4.7	322	
600 min Summer	7.826	0.0	5.0	390	
720 min Summer	6.878	0.0	5.3	458	
960 min Summer	5.608	0.0	5.7	590	
1440 min Summer	4.204	0.0	6.4	852	
2160 min Summer	3.150	0.0	7.5	1232	
2880 min Summer	2.565	0.0	8.1	1588	
4320 min Summer	1.917	0.0	9.0	2336	
5760 min Summer	1.558	0.0	10.0	3056	
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Ormond House Upper Ormond Quay Dublin 7						
Date 23/02/2023 20:57 File 20220829 Cascade.CASX		Designed by besterd Checked by				
Innovyze			Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P6).SRCX</u>						
Storm Event		Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Summer		22.569	0.019	0.1	2.1	Flood Risk
8640 min Summer		22.568	0.018	0.1	2.0	Flood Risk
10080 min Summer		22.567	0.017	0.1	1.9	Flood Risk
15 min Winter		22.564	0.014	0.0	1.6	Flood Risk
30 min Winter		22.569	0.019	0.1	2.1	Flood Risk
60 min Winter		22.574	0.024	0.1	2.7	Flood Risk
120 min Winter		22.579	0.029	0.1	3.2	Flood Risk
180 min Winter		22.580	0.030	0.1	3.4	Flood Risk
240 min Winter		22.581	0.031	0.1	3.5	Flood Risk
360 min Winter		22.583	0.033	0.2	3.6	Flood Risk
480 min Winter		22.583	0.033	0.2	3.7	Flood Risk
600 min Winter		22.583	0.033	0.2	3.7	Flood Risk
720 min Winter		22.583	0.033	0.2	3.6	Flood Risk
960 min Winter		22.582	0.032	0.1	3.5	Flood Risk
1440 min Winter		22.580	0.030	0.1	3.3	Flood Risk
2160 min Winter		22.577	0.027	0.1	2.9	Flood Risk
2880 min Winter		22.574	0.024	0.1	2.7	Flood Risk
4320 min Winter		22.571	0.021	0.1	2.3	Flood Risk
5760 min Winter		22.569	0.019	0.1	2.1	Flood Risk
Storm Event		Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Summer		1.326	0.0	10.6	3752	
8640 min Summer		1.162	0.0	11.1	4496	
10080 min Summer		1.039	0.0	11.5	5240	
15 min Winter		84.523	0.0	1.2	19	
30 min Winter		58.468	0.0	1.8	33	
60 min Winter		38.020	0.0	2.6	62	
120 min Winter		23.997	0.0	3.4	118	
180 min Winter		18.178	0.0	3.9	172	
240 min Winter		14.896	0.0	4.2	196	
360 min Winter		11.219	0.0	4.8	270	
480 min Winter		9.163	0.0	5.3	346	
600 min Winter		7.826	0.0	5.6	420	
720 min Winter		6.878	0.0	5.9	492	
960 min Winter		5.608	0.0	6.4	634	
1440 min Winter		4.204	0.0	7.2	898	
2160 min Winter		3.150	0.0	8.4	1280	
2880 min Winter		2.565	0.0	9.1	1644	
4320 min Winter		1.917	0.0	10.1	2380	
5760 min Winter		1.558	0.0	11.2	3112	
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Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P6).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Winter	22.567	0.017	0.1	1.9	Flood Risk
8640 min Winter	22.566	0.016	0.1	1.7	Flood Risk
10080 min Winter	22.565	0.015	0.1	1.6	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Winter	1.326	0.0	11.9	3896	
8640 min Winter	1.162	0.0	12.5	4584	
10080 min Winter	1.039	0.0	12.9	5344	
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Ormond House Upper Ormond Quay Dublin 7																																																																																																																																	
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<u>Cascade Model Details for 220830 GreenBlue Podium (P6).SRCX</u> Storage is Online Cover Level (m) 22.750 <u>Tank or Pond Structure</u> Invert Level (m) 22.550 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>111.0</td><td>0.108</td><td>111.0</td><td>0.109</td><td>0.0</td></tr></table> <u>Hydro-Brake® Optimum Outflow Control</u> <table><tr><td>Unit Reference</td><td>MD-SHE-0028-2000-0109-2000</td></tr><tr><td>Design Head (m)</td><td>0.109</td></tr><tr><td>Design Flow (l/s)</td><td>0.2</td></tr><tr><td>Flush-Flo™</td><td>Calculated</td></tr><tr><td>Objective</td><td>Minimise upstream storage</td></tr><tr><td>Application</td><td>Surface</td></tr><tr><td>Sump Available</td><td>Yes</td></tr><tr><td>Diameter (mm)</td><td>28</td></tr><tr><td>Invert Level (m)</td><td>22.550</td></tr><tr><td>Minimum Outlet Pipe Diameter (mm)</td><td>75</td></tr><tr><td>Suggested Manhole Diameter (mm)</td><td>1200</td></tr></table> <table><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr><tr><td>Design Point (Calculated)</td><td>0.109</td><td>0.2</td><td>Kick-Flo®</td><td>0.079</td><td>0.2</td></tr><tr><td>Flush-Flo™</td><td>0.046</td><td>0.2</td><td>Mean Flow over Head Range</td><td>-</td><td>0.2</td></tr></table> The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated <table><tr><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th></tr><tr><td>0.100</td><td>0.2</td><td>1.200</td><td>0.6</td><td>3.000</td><td>0.9</td><td>7.000</td><td>1.3</td></tr><tr><td>0.200</td><td>0.3</td><td>1.400</td><td>0.6</td><td>3.500</td><td>1.0</td><td>7.500</td><td>1.4</td></tr><tr><td>0.300</td><td>0.3</td><td>1.600</td><td>0.6</td><td>4.000</td><td>1.0</td><td>8.000</td><td>1.4</td></tr><tr><td>0.400</td><td>0.3</td><td>1.800</td><td>0.7</td><td>4.500</td><td>1.1</td><td>8.500</td><td>1.5</td></tr><tr><td>0.500</td><td>0.4</td><td>2.000</td><td>0.7</td><td>5.000</td><td>1.1</td><td>9.000</td><td>1.5</td></tr><tr><td>0.600</td><td>0.4</td><td>2.200</td><td>0.8</td><td>5.500</td><td>1.2</td><td>9.500</td><td>1.6</td></tr><tr><td>0.800</td><td>0.5</td><td>2.400</td><td>0.8</td><td>6.000</td><td>1.2</td><td></td><td></td></tr><tr><td>1.000</td><td>0.5</td><td>2.600</td><td>0.8</td><td>6.500</td><td>1.3</td><td></td><td></td></tr></table>						Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	0.000	111.0	0.108	111.0	0.109	0.0	Unit Reference	MD-SHE-0028-2000-0109-2000	Design Head (m)	0.109	Design Flow (l/s)	0.2	Flush-Flo™	Calculated	Objective	Minimise upstream storage	Application	Surface	Sump Available	Yes	Diameter (mm)	28	Invert Level (m)	22.550	Minimum Outlet Pipe Diameter (mm)	75	Suggested Manhole Diameter (mm)	1200	Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	0.109	0.2	Kick-Flo®	0.079	0.2	Flush-Flo™	0.046	0.2	Mean Flow over Head Range	-	0.2	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	0.2	1.200	0.6	3.000	0.9	7.000	1.3	0.200	0.3	1.400	0.6	3.500	1.0	7.500	1.4	0.300	0.3	1.600	0.6	4.000	1.0	8.000	1.4	0.400	0.3	1.800	0.7	4.500	1.1	8.500	1.5	0.500	0.4	2.000	0.7	5.000	1.1	9.000	1.5	0.600	0.4	2.200	0.8	5.500	1.2	9.500	1.6	0.800	0.5	2.400	0.8	6.000	1.2			1.000	0.5	2.600	0.8	6.500	1.3		
Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)																																																																																																																												
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Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P7).SRCX</u>					
Upstream Structures		Outflow To		Overflow To	
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	22.564	0.014	0.0	2.8	Flood Risk
30 min Summer	22.569	0.019	0.1	3.9	Flood Risk
60 min Summer	22.574	0.024	0.1	4.9	Flood Risk
120 min Summer	22.579	0.029	0.1	6.0	Flood Risk
180 min Summer	22.582	0.032	0.1	6.6	Flood Risk
240 min Summer	22.583	0.033	0.2	6.9	Flood Risk
360 min Summer	22.585	0.035	0.2	7.3	Flood Risk
480 min Summer	22.586	0.036	0.2	7.6	Flood Risk
600 min Summer	22.587	0.037	0.2	7.8	Flood Risk
720 min Summer	22.588	0.038	0.2	7.9	Flood Risk
960 min Summer	22.589	0.039	0.2	8.1	Flood Risk
1440 min Summer	22.589	0.039	0.2	8.1	Flood Risk
2160 min Summer	22.588	0.038	0.2	8.0	Flood Risk
2880 min Summer	22.587	0.037	0.2	7.7	Flood Risk
4320 min Summer	22.584	0.034	0.2	7.0	Flood Risk
5760 min Summer	22.581	0.031	0.1	6.4	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	1.7	19	
30 min Summer	58.468	0.0	2.6	34	
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120 min Summer	23.997	0.0	5.6	122	
180 min Summer	18.178	0.0	6.5	182	
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360 min Summer	11.219	0.0	8.1	314	
480 min Summer	9.163	0.0	8.9	376	
600 min Summer	7.826	0.0	9.5	436	
720 min Summer	6.878	0.0	10.0	506	
960 min Summer	5.608	0.0	10.9	642	
1440 min Summer	4.204	0.0	12.1	910	
2160 min Summer	3.150	0.0	14.7	1316	
2880 min Summer	2.565	0.0	15.9	1704	
4320 min Summer	1.917	0.0	17.6	2464	
5760 min Summer	1.558	0.0	19.8	3224	
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<u>Cascade Summary of Results for 220830 GreenBlue Podium (P7).SRCX</u>																																																																																																																														
<table><thead><tr><th>Storm Event</th><th>Max Level (m)</th><th>Max Depth (m)</th><th>Max Control (l/s)</th><th>Max Volume (m³)</th><th>Status</th></tr></thead><tbody><tr><td>7200 min Summer</td><td>22.579</td><td>0.029</td><td>0.1</td><td>6.0</td><td>Flood Risk</td></tr><tr><td>8640 min Summer</td><td>22.577</td><td>0.027</td><td>0.1</td><td>5.6</td><td>Flood Risk</td></tr><tr><td>10080 min Summer</td><td>22.575</td><td>0.025</td><td>0.1</td><td>5.3</td><td>Flood Risk</td></tr><tr><td>15 min Winter</td><td>22.565</td><td>0.015</td><td>0.1</td><td>3.2</td><td>Flood Risk</td></tr><tr><td>30 min Winter</td><td>22.571</td><td>0.021</td><td>0.1</td><td>4.3</td><td>Flood Risk</td></tr><tr><td>60 min Winter</td><td>22.577</td><td>0.027</td><td>0.1</td><td>5.5</td><td>Flood Risk</td></tr><tr><td>120 min Winter</td><td>22.582</td><td>0.032</td><td>0.2</td><td>6.7</td><td>Flood Risk</td></tr><tr><td>180 min Winter</td><td>22.586</td><td>0.036</td><td>0.2</td><td>7.4</td><td>Flood Risk</td></tr><tr><td>240 min Winter</td><td>22.587</td><td>0.037</td><td>0.2</td><td>7.8</td><td>Flood Risk</td></tr><tr><td>360 min Winter</td><td>22.590</td><td>0.040</td><td>0.2</td><td>8.2</td><td>Flood Risk</td></tr><tr><td>480 min Winter</td><td>22.591</td><td>0.041</td><td>0.2</td><td>8.5</td><td>Flood Risk</td></tr><tr><td>600 min Winter</td><td>22.592</td><td>0.042</td><td>0.2</td><td>8.7</td><td>Flood Risk</td></tr><tr><td>720 min Winter</td><td>22.592</td><td>0.042</td><td>0.2</td><td>8.8</td><td>Flood Risk</td></tr><tr><td>960 min Winter</td><td>22.592</td><td>0.042</td><td>0.2</td><td>8.8</td><td>Flood Risk</td></tr><tr><td>1440 min Winter</td><td>22.592</td><td>0.042</td><td>0.2</td><td>8.7</td><td>Flood Risk</td></tr><tr><td>2160 min Winter</td><td>22.589</td><td>0.039</td><td>0.2</td><td>8.2</td><td>Flood Risk</td></tr><tr><td>2880 min Winter</td><td>22.587</td><td>0.037</td><td>0.2</td><td>7.7</td><td>Flood Risk</td></tr><tr><td>4320 min Winter</td><td>22.582</td><td>0.032</td><td>0.2</td><td>6.7</td><td>Flood Risk</td></tr><tr><td>5760 min Winter</td><td>22.579</td><td>0.029</td><td>0.1</td><td>6.0</td><td>Flood Risk</td></tr></tbody></table>							Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status	7200 min Summer	22.579	0.029	0.1	6.0	Flood Risk	8640 min Summer	22.577	0.027	0.1	5.6	Flood Risk	10080 min Summer	22.575	0.025	0.1	5.3	Flood Risk	15 min Winter	22.565	0.015	0.1	3.2	Flood Risk	30 min Winter	22.571	0.021	0.1	4.3	Flood Risk	60 min Winter	22.577	0.027	0.1	5.5	Flood Risk	120 min Winter	22.582	0.032	0.2	6.7	Flood Risk	180 min Winter	22.586	0.036	0.2	7.4	Flood Risk	240 min Winter	22.587	0.037	0.2	7.8	Flood Risk	360 min Winter	22.590	0.040	0.2	8.2	Flood Risk	480 min Winter	22.591	0.041	0.2	8.5	Flood Risk	600 min Winter	22.592	0.042	0.2	8.7	Flood Risk	720 min Winter	22.592	0.042	0.2	8.8	Flood Risk	960 min Winter	22.592	0.042	0.2	8.8	Flood Risk	1440 min Winter	22.592	0.042	0.2	8.7	Flood Risk	2160 min Winter	22.589	0.039	0.2	8.2	Flood Risk	2880 min Winter	22.587	0.037	0.2	7.7	Flood Risk	4320 min Winter	22.582	0.032	0.2	6.7	Flood Risk	5760 min Winter	22.579	0.029	0.1	6.0	Flood Risk
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15 min Winter	22.565	0.015	0.1	3.2	Flood Risk																																																																																																																									
30 min Winter	22.571	0.021	0.1	4.3	Flood Risk																																																																																																																									
60 min Winter	22.577	0.027	0.1	5.5	Flood Risk																																																																																																																									
120 min Winter	22.582	0.032	0.2	6.7	Flood Risk																																																																																																																									
180 min Winter	22.586	0.036	0.2	7.4	Flood Risk																																																																																																																									
240 min Winter	22.587	0.037	0.2	7.8	Flood Risk																																																																																																																									
360 min Winter	22.590	0.040	0.2	8.2	Flood Risk																																																																																																																									
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5760 min Winter	22.579	0.029	0.1	6.0	Flood Risk																																																																																																																									
<table><thead><tr><th>Storm Event</th><th>Rain (mm/hr)</th><th>Flooded Volume (m³)</th><th>Discharge Volume (m³)</th><th>Time-Peak (mins)</th></tr></thead><tbody><tr><td>7200 min Summer</td><td>1.326</td><td>0.0</td><td>21.0</td><td>3960</td></tr><tr><td>8640 min Summer</td><td>1.162</td><td>0.0</td><td>22.0</td><td>4672</td></tr><tr><td>10080 min Summer</td><td>1.039</td><td>0.0</td><td>22.7</td><td>5352</td></tr><tr><td>15 min Winter</td><td>84.523</td><td>0.0</td><td>2.0</td><td>19</td></tr><tr><td>30 min Winter</td><td>58.468</td><td>0.0</td><td>3.1</td><td>33</td></tr><tr><td>60 min Winter</td><td>38.020</td><td>0.0</td><td>4.9</td><td>62</td></tr><tr><td>120 min Winter</td><td>23.997</td><td>0.0</td><td>6.4</td><td>120</td></tr><tr><td>180 min Winter</td><td>18.178</td><td>0.0</td><td>7.3</td><td>178</td></tr><tr><td>240 min Winter</td><td>14.896</td><td>0.0</td><td>8.0</td><td>234</td></tr><tr><td>360 min Winter</td><td>11.219</td><td>0.0</td><td>9.2</td><td>340</td></tr><tr><td>480 min Winter</td><td>9.163</td><td>0.0</td><td>10.0</td><td>388</td></tr><tr><td>600 min Winter</td><td>7.826</td><td>0.0</td><td>10.7</td><td>462</td></tr><tr><td>720 min Winter</td><td>6.878</td><td>0.0</td><td>11.3</td><td>538</td></tr><tr><td>960 min Winter</td><td>5.608</td><td>0.0</td><td>12.3</td><td>692</td></tr><tr><td>1440 min Winter</td><td>4.204</td><td>0.0</td><td>13.7</td><td>982</td></tr><tr><td>2160 min Winter</td><td>3.150</td><td>0.0</td><td>16.5</td><td>1404</td></tr><tr><td>2880 min Winter</td><td>2.565</td><td>0.0</td><td>17.9</td><td>1812</td></tr><tr><td>4320 min Winter</td><td>1.917</td><td>0.0</td><td>19.8</td><td>2592</td></tr><tr><td>5760 min Winter</td><td>1.558</td><td>0.0</td><td>22.2</td><td>3296</td></tr></tbody></table>							Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	7200 min Summer	1.326	0.0	21.0	3960	8640 min Summer	1.162	0.0	22.0	4672	10080 min Summer	1.039	0.0	22.7	5352	15 min Winter	84.523	0.0	2.0	19	30 min Winter	58.468	0.0	3.1	33	60 min Winter	38.020	0.0	4.9	62	120 min Winter	23.997	0.0	6.4	120	180 min Winter	18.178	0.0	7.3	178	240 min Winter	14.896	0.0	8.0	234	360 min Winter	11.219	0.0	9.2	340	480 min Winter	9.163	0.0	10.0	388	600 min Winter	7.826	0.0	10.7	462	720 min Winter	6.878	0.0	11.3	538	960 min Winter	5.608	0.0	12.3	692	1440 min Winter	4.204	0.0	13.7	982	2160 min Winter	3.150	0.0	16.5	1404	2880 min Winter	2.565	0.0	17.9	1812	4320 min Winter	1.917	0.0	19.8	2592	5760 min Winter	1.558	0.0	22.2	3296																				
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 20:58		Designed by besterd			
File 20220829 Cascade.CASX		Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P7).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Winter	22.576	0.026	0.1	5.4	Flood Risk
8640 min Winter	22.574	0.024	0.1	5.0	Flood Risk
10080 min Winter	22.572	0.022	0.1	4.7	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Winter	1.326	0.0	23.6	4032	
8640 min Winter	1.162	0.0	24.7	4760	
10080 min Winter	1.039	0.0	25.6	5544	
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<u>Cascade Model Details for 220830 GreenBlue Podium (P7).SRCX</u> Storage is Online Cover Level (m) 22.750 <u>Tank or Pond Structure</u> Invert Level (m) 22.550 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>208.2</td><td>0.108</td><td>208.2</td><td>0.109</td><td>0.0</td></tr></table> <u>Hydro-Brake® Optimum Outflow Control</u> <table><tr><td>Unit Reference</td><td>MD-SHE-0028-2000-0109-2000</td></tr><tr><td>Design Head (m)</td><td>0.109</td></tr><tr><td>Design Flow (l/s)</td><td>0.2</td></tr><tr><td>Flush-Flo™</td><td>Calculated</td></tr><tr><td>Objective</td><td>Minimise upstream storage</td></tr><tr><td>Application</td><td>Surface</td></tr><tr><td>Sump Available</td><td>Yes</td></tr><tr><td>Diameter (mm)</td><td>28</td></tr><tr><td>Invert Level (m)</td><td>22.550</td></tr><tr><td>Minimum Outlet Pipe Diameter (mm)</td><td>75</td></tr><tr><td>Suggested Manhole Diameter (mm)</td><td>1200</td></tr></table> <table><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr><tr><td>Design Point (Calculated)</td><td>0.109</td><td>0.2</td><td>Kick-Flo®</td><td>0.079</td><td>0.2</td></tr><tr><td>Flush-Flo™</td><td>0.046</td><td>0.2</td><td>Mean Flow over Head Range</td><td>-</td><td>0.2</td></tr></table> The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated <table><tr><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th></tr><tr><td>0.100</td><td>0.2</td><td>1.200</td><td>0.6</td><td>3.000</td><td>0.9</td><td>7.000</td><td>1.3</td></tr><tr><td>0.200</td><td>0.3</td><td>1.400</td><td>0.6</td><td>3.500</td><td>1.0</td><td>7.500</td><td>1.4</td></tr><tr><td>0.300</td><td>0.3</td><td>1.600</td><td>0.6</td><td>4.000</td><td>1.0</td><td>8.000</td><td>1.4</td></tr><tr><td>0.400</td><td>0.3</td><td>1.800</td><td>0.7</td><td>4.500</td><td>1.1</td><td>8.500</td><td>1.5</td></tr><tr><td>0.500</td><td>0.4</td><td>2.000</td><td>0.7</td><td>5.000</td><td>1.1</td><td>9.000</td><td>1.5</td></tr><tr><td>0.600</td><td>0.4</td><td>2.200</td><td>0.8</td><td>5.500</td><td>1.2</td><td>9.500</td><td>1.6</td></tr><tr><td>0.800</td><td>0.5</td><td>2.400</td><td>0.8</td><td>6.000</td><td>1.2</td><td></td><td></td></tr><tr><td>1.000</td><td>0.5</td><td>2.600</td><td>0.8</td><td>6.500</td><td>1.3</td><td></td><td></td></tr></table>						Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	0.000	208.2	0.108	208.2	0.109	0.0	Unit Reference	MD-SHE-0028-2000-0109-2000	Design Head (m)	0.109	Design Flow (l/s)	0.2	Flush-Flo™	Calculated	Objective	Minimise upstream storage	Application	Surface	Sump Available	Yes	Diameter (mm)	28	Invert Level (m)	22.550	Minimum Outlet Pipe Diameter (mm)	75	Suggested Manhole Diameter (mm)	1200	Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	0.109	0.2	Kick-Flo®	0.079	0.2	Flush-Flo™	0.046	0.2	Mean Flow over Head Range	-	0.2	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	0.2	1.200	0.6	3.000	0.9	7.000	1.3	0.200	0.3	1.400	0.6	3.500	1.0	7.500	1.4	0.300	0.3	1.600	0.6	4.000	1.0	8.000	1.4	0.400	0.3	1.800	0.7	4.500	1.1	8.500	1.5	0.500	0.4	2.000	0.7	5.000	1.1	9.000	1.5	0.600	0.4	2.200	0.8	5.500	1.2	9.500	1.6	0.800	0.5	2.400	0.8	6.000	1.2			1.000	0.5	2.600	0.8	6.500	1.3		
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 20:58		Designed by besterd			
File 20220829 Cascade.CASX		Checked by			
Innovyze			Source Control 2020.1		
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P8).SRCX</u>					
Upstream Structures	Outflow To		Overflow To		
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	9.214	0.014	0.0	4.9	Flood Risk
30 min Summer	9.219	0.019	0.1	6.7	Flood Risk
60 min Summer	9.225	0.025	0.1	8.6	Flood Risk
120 min Summer	9.230	0.030	0.1	10.7	Flood Risk
180 min Summer	9.234	0.034	0.2	11.9	Flood Risk
240 min Summer	9.236	0.036	0.2	12.7	Flood Risk
360 min Summer	9.239	0.039	0.2	13.7	Flood Risk
480 min Summer	9.241	0.041	0.2	14.3	Flood Risk
600 min Summer	9.242	0.042	0.2	14.7	Flood Risk
720 min Summer	9.243	0.043	0.2	15.0	Flood Risk
960 min Summer	9.244	0.044	0.2	15.5	Flood Risk
1440 min Summer	9.246	0.046	0.2	16.0	Flood Risk
2160 min Summer	9.247	0.047	0.2	16.3	Flood Risk
2880 min Summer	9.246	0.046	0.2	16.2	Flood Risk
4320 min Summer	9.244	0.044	0.2	15.5	Flood Risk
5760 min Summer	9.242	0.042	0.2	14.7	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	2.3	19	
30 min Summer	58.468	0.0	3.7	34	
60 min Summer	38.020	0.0	6.7	64	
120 min Summer	23.997	0.0	8.8	122	
180 min Summer	18.178	0.0	10.2	182	
240 min Summer	14.896	0.0	11.2	242	
360 min Summer	11.219	0.0	12.8	360	
480 min Summer	9.163	0.0	14.0	480	
600 min Summer	7.826	0.0	14.9	546	
720 min Summer	6.878	0.0	15.7	598	
960 min Summer	5.608	0.0	17.0	720	
1440 min Summer	4.204	0.0	18.6	982	
2160 min Summer	3.150	0.0	24.4	1404	
2880 min Summer	2.565	0.0	26.4	1816	
4320 min Summer	1.917	0.0	28.9	2596	
5760 min Summer	1.558	0.0	33.7	3392	
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Ormond House Upper Ormond Quay Dublin 7																																																																																																																														
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 20:58		Designed by besterd			
File 20220829 Cascade.CASX		Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Podium (P8).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Winter	9.238	0.038	0.2	13.4	Flood Risk
8640 min Winter	9.235	0.035	0.2	12.4	Flood Risk
10080 min Winter	9.233	0.033	0.2	11.5	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Winter	1.326	0.0	40.2	4328	
8640 min Winter	1.162	0.0	42.0	5096	
10080 min Winter	1.039	0.0	43.3	5848	
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 21:00 File 20220829 Cascade.CASX		Designed by besterd Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Roof (G1).SRCX</u>					
Upstream Structures		Outflow To		Overflow To	
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	23.389	0.014	0.0	4.9	Flood Risk
30 min Summer	23.394	0.019	0.1	6.7	Flood Risk
60 min Summer	23.399	0.024	0.1	8.6	Flood Risk
120 min Summer	23.405	0.030	0.1	10.7	Flood Risk
180 min Summer	23.408	0.033	0.2	11.9	Flood Risk
240 min Summer	23.411	0.036	0.2	12.7	Flood Risk
360 min Summer	23.413	0.038	0.2	13.7	Flood Risk
480 min Summer	23.415	0.040	0.2	14.3	Flood Risk
600 min Summer	23.416	0.041	0.2	14.7	Flood Risk
720 min Summer	23.417	0.042	0.2	15.0	Flood Risk
960 min Summer	23.418	0.043	0.2	15.5	Flood Risk
1440 min Summer	23.420	0.045	0.2	16.1	Flood Risk
2160 min Summer	23.421	0.046	0.2	16.4	Flood Risk
2880 min Summer	23.421	0.046	0.2	16.3	Flood Risk
4320 min Summer	23.419	0.044	0.2	15.6	Flood Risk
5760 min Summer	23.417	0.042	0.2	14.8	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	2.3	19	
30 min Summer	58.468	0.0	3.6	34	
60 min Summer	38.020	0.0	6.6	64	
120 min Summer	23.997	0.0	8.7	122	
180 min Summer	18.178	0.0	10.1	182	
240 min Summer	14.896	0.0	11.1	242	
360 min Summer	11.219	0.0	12.7	362	
480 min Summer	9.163	0.0	13.9	480	
600 min Summer	7.826	0.0	14.8	548	
720 min Summer	6.878	0.0	15.6	600	
960 min Summer	5.608	0.0	16.9	722	
1440 min Summer	4.204	0.0	18.5	982	
2160 min Summer	3.150	0.0	24.3	1404	
2880 min Summer	2.565	0.0	26.3	1812	
4320 min Summer	1.917	0.0	28.7	2596	
5760 min Summer	1.558	0.0	33.7	3400	
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 21:00 File 20220829 Cascade.CASX		Designed by besterd Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Roof (G1).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Summer	23.414	0.039	0.2	14.1	Flood Risk
8640 min Summer	23.412	0.037	0.2	13.3	Flood Risk
10080 min Summer	23.410	0.035	0.2	12.7	Flood Risk
15 min Winter	23.390	0.015	0.1	5.5	Flood Risk
30 min Winter	23.396	0.021	0.1	7.5	Flood Risk
60 min Winter	23.402	0.027	0.1	9.7	Flood Risk
120 min Winter	23.408	0.033	0.2	12.0	Flood Risk
180 min Winter	23.412	0.037	0.2	13.3	Flood Risk
240 min Winter	23.415	0.040	0.2	14.3	Flood Risk
360 min Winter	23.418	0.043	0.2	15.5	Flood Risk
480 min Winter	23.420	0.045	0.2	16.2	Flood Risk
600 min Winter	23.422	0.047	0.2	16.7	Flood Risk
720 min Winter	23.423	0.048	0.2	17.0	Flood Risk
960 min Winter	23.424	0.049	0.2	17.4	Flood Risk
1440 min Winter	23.425	0.050	0.2	17.9	Flood Risk
2160 min Winter	23.425	0.050	0.2	17.9	Flood Risk
2880 min Winter	23.424	0.049	0.2	17.4	Flood Risk
4320 min Winter	23.420	0.045	0.2	16.0	Flood Risk
5760 min Winter	23.416	0.041	0.2	14.7	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Summer	1.326	0.0	35.7	4176	
8640 min Summer	1.162	0.0	37.3	4920	
10080 min Summer	1.039	0.0	38.4	5648	
15 min Winter	84.523	0.0	2.7	19	
30 min Winter	58.468	0.0	4.2	33	
60 min Winter	38.020	0.0	7.6	62	
120 min Winter	23.997	0.0	9.9	120	
180 min Winter	18.178	0.0	11.5	180	
240 min Winter	14.896	0.0	12.6	238	
360 min Winter	11.219	0.0	14.4	352	
480 min Winter	9.163	0.0	15.7	462	
600 min Winter	7.826	0.0	16.8	572	
720 min Winter	6.878	0.0	17.6	672	
960 min Winter	5.608	0.0	19.0	768	
1440 min Winter	4.204	0.0	20.7	1080	
2160 min Winter	3.150	0.0	27.4	1536	
2880 min Winter	2.565	0.0	29.6	1960	
4320 min Winter	1.917	0.0	32.4	2768	
5760 min Winter	1.558	0.0	37.8	3576	
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 21:00		Designed by besterd			
File 20220829 Cascade.CASX		Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Roof (G1).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Winter	23.413	0.038	0.2	13.6	Flood Risk
8640 min Winter	23.410	0.035	0.2	12.6	Flood Risk
10080 min Winter	23.408	0.033	0.2	11.7	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Winter	1.326	0.0	40.1	4328	
8640 min Winter	1.162	0.0	41.9	5096	
10080 min Winter	1.039	0.0	43.2	5848	
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 21:00 File 20220829 Cascade.CASX		Designed by besterd Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Roof (G2).SRCX</u>					
Upstream Structures		Outflow To		Overflow To	
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	38.689	0.014	0.0	6.8	Flood Risk
30 min Summer	38.695	0.020	0.1	9.4	Flood Risk
60 min Summer	38.700	0.025	0.1	12.1	Flood Risk
120 min Summer	38.706	0.031	0.1	15.0	Flood Risk
180 min Summer	38.710	0.035	0.2	16.7	Flood Risk
240 min Summer	38.713	0.038	0.2	18.0	Flood Risk
360 min Summer	38.716	0.041	0.2	19.7	Flood Risk
480 min Summer	38.718	0.043	0.2	20.8	Flood Risk
600 min Summer	38.720	0.045	0.2	21.5	Flood Risk
720 min Summer	38.721	0.046	0.2	22.0	Flood Risk
960 min Summer	38.723	0.048	0.2	22.8	Flood Risk
1440 min Summer	38.725	0.050	0.2	23.9	Flood Risk
2160 min Summer	38.727	0.052	0.2	24.8	Flood Risk
2880 min Summer	38.727	0.052	0.2	25.0	Flood Risk
4320 min Summer	38.727	0.052	0.2	24.7	Flood Risk
5760 min Summer	38.725	0.050	0.2	23.8	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	2.7	19	
30 min Summer	58.468	0.0	4.3	34	
60 min Summer	38.020	0.0	8.5	64	
120 min Summer	23.997	0.0	11.2	124	
180 min Summer	18.178	0.0	13.0	182	
240 min Summer	14.896	0.0	14.3	242	
360 min Summer	11.219	0.0	16.3	362	
480 min Summer	9.163	0.0	17.8	482	
600 min Summer	7.826	0.0	18.9	600	
720 min Summer	6.878	0.0	19.8	716	
960 min Summer	5.608	0.0	21.2	828	
1440 min Summer	4.204	0.0	22.5	1094	
2160 min Summer	3.150	0.0	32.5	1496	
2880 min Summer	2.565	0.0	34.9	1908	
4320 min Summer	1.917	0.0	37.4	2724	
5760 min Summer	1.558	0.0	46.2	3520	
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DBFL Consulting Engineers					Page 2																																																																																																																								
Ormond House Upper Ormond Quay Dublin 7																																																																																																																													
Date 23/02/2023 21:00 File 20220829 Cascade.CASX																																																																																																																													
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Ormond House Upper Ormond Quay Dublin 7					
Date 23/02/2023 21:00		Designed by besterd			
File 20220829 Cascade.CASX		Checked by			
Innovyze		Source Control 2020.1			
<u>Cascade Summary of Results for 220830 GreenBlue Roof (G2).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Winter	38.723	0.048	0.2	23.0	Flood Risk
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7200 min Winter	1.326	0.0	54.9	4544	
8640 min Winter	1.162	0.0	57.3	5280	
10080 min Winter	1.039	0.0	58.9	6056	
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Design Head (m) 0.108																																																																													
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Objective Minimise upstream storage																																																																													
Application Surface																																																																													
Sump Available Yes																																																																													
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Minimum Outlet Pipe Diameter (mm) 75																																																																													
Suggested Manhole Diameter (mm) 1200																																																																													
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Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)																																																																						
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<u>Cascade Summary of Results for 220830 GreenBlue Roof (G3).SRCX</u>					
Upstream Structures		Outflow To		Overflow To	
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	32.014	0.014	0.0	6.9	Flood Risk
30 min Summer	32.019	0.019	0.1	9.6	Flood Risk
60 min Summer	32.025	0.025	0.1	12.3	Flood Risk
120 min Summer	32.031	0.031	0.1	15.3	Flood Risk
180 min Summer	32.035	0.035	0.2	17.1	Flood Risk
240 min Summer	32.037	0.037	0.2	18.5	Flood Risk
360 min Summer	32.041	0.041	0.2	20.2	Flood Risk
480 min Summer	32.043	0.043	0.2	21.4	Flood Risk
600 min Summer	32.045	0.045	0.2	22.1	Flood Risk
720 min Summer	32.046	0.046	0.2	22.7	Flood Risk
960 min Summer	32.048	0.048	0.2	23.5	Flood Risk
1440 min Summer	32.050	0.050	0.2	24.6	Flood Risk
2160 min Summer	32.052	0.052	0.2	25.5	Flood Risk
2880 min Summer	32.052	0.052	0.2	25.8	Flood Risk
4320 min Summer	32.052	0.052	0.2	25.6	Flood Risk
5760 min Summer	32.050	0.050	0.2	24.7	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	2.7	19	
30 min Summer	58.468	0.0	4.4	34	
60 min Summer	38.020	0.0	8.6	64	
120 min Summer	23.997	0.0	11.3	124	
180 min Summer	18.178	0.0	13.1	182	
240 min Summer	14.896	0.0	14.5	242	
360 min Summer	11.219	0.0	16.5	362	
480 min Summer	9.163	0.0	18.0	482	
600 min Summer	7.826	0.0	19.1	600	
720 min Summer	6.878	0.0	20.1	720	
960 min Summer	5.608	0.0	21.4	838	
1440 min Summer	4.204	0.0	22.6	1108	
2160 min Summer	3.150	0.0	33.1	1512	
2880 min Summer	2.565	0.0	35.5	1928	
4320 min Summer	1.917	0.0	37.9	2728	
5760 min Summer	1.558	0.0	47.1	3528	
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<u>Cascade Summary of Results for 220830 GreenBlue Roof (G3).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Summer	32.048	0.048	0.2	23.7	Flood Risk
8640 min Summer	32.046	0.046	0.2	22.7	Flood Risk
10080 min Summer	32.044	0.044	0.2	21.8	Flood Risk
15 min Winter	32.016	0.016	0.1	7.8	Flood Risk
30 min Winter	32.022	0.022	0.1	10.7	Flood Risk
60 min Winter	32.028	0.028	0.1	13.8	Flood Risk
120 min Winter	32.035	0.035	0.2	17.2	Flood Risk
180 min Winter	32.039	0.039	0.2	19.2	Flood Risk
240 min Winter	32.042	0.042	0.2	20.7	Flood Risk
360 min Winter	32.046	0.046	0.2	22.7	Flood Risk
480 min Winter	32.049	0.049	0.2	24.1	Flood Risk
600 min Winter	32.051	0.051	0.2	25.1	Flood Risk
720 min Winter	32.052	0.052	0.2	25.9	Flood Risk
960 min Winter	32.055	0.055	0.2	26.9	Flood Risk
1440 min Winter	32.057	0.057	0.2	27.9	Flood Risk
2160 min Winter	32.058	0.058	0.2	28.7	Flood Risk
2880 min Winter	32.058	0.058	0.2	28.7	Flood Risk
4320 min Winter	32.056	0.056	0.2	27.5	Flood Risk
5760 min Winter	32.052	0.052	0.2	25.8	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Summer	1.326	0.0	49.9	4320	
8640 min Summer	1.162	0.0	52.0	5096	
10080 min Summer	1.039	0.0	53.3	5848	
15 min Winter	84.523	0.0	3.2	19	
30 min Winter	58.468	0.0	5.1	34	
60 min Winter	38.020	0.0	9.8	64	
120 min Winter	23.997	0.0	12.9	122	
180 min Winter	18.178	0.0	14.9	180	
240 min Winter	14.896	0.0	16.5	238	
360 min Winter	11.219	0.0	18.7	354	
480 min Winter	9.163	0.0	20.3	470	
600 min Winter	7.826	0.0	21.5	584	
720 min Winter	6.878	0.0	22.4	696	
960 min Winter	5.608	0.0	23.7	916	
1440 min Winter	4.204	0.0	24.6	1298	
2160 min Winter	3.150	0.0	37.1	1644	
2880 min Winter	2.565	0.0	39.7	2104	
4320 min Winter	1.917	0.0	42.2	2984	
5760 min Winter	1.558	0.0	53.0	3808	
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<u>Cascade Summary of Results for 220830 GreenBlue Roof (G3).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Winter	32.049	0.049	0.2	24.0	Flood Risk
8640 min Winter	32.045	0.045	0.2	22.4	Flood Risk
10080 min Winter	32.043	0.043	0.2	21.1	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Winter	1.326	0.0	56.1	4608	
8640 min Winter	1.162	0.0	58.5	5360	
10080 min Winter	1.039	0.0	60.1	6144	
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Cascade Model Details for 220830 GreenBlue Roof (G3).SRCX

Storage is Online Cover Level (m) 32.200

Tank or Pond Structure

Invert Level (m) 32.000

Depth (m)	Area (m²)	Depth (m)	Area (m²)	Depth (m)	Area (m²)
0.000	493.8	0.108	493.8	0.109	0.0

Hydro-Brake® Optimum Outflow Control

Unit Reference MD-SHE-0028-2000-0108-2000

Design Head (m) 0.108

Design Flow (l/s) 0.2

Flush-Flo™ Calculated

Objective Minimise upstream storage

Application Surface

Sump Available Yes

Diameter (mm) 28

Invert Level (m) 32.000

Minimum Outlet Pipe Diameter (mm) 75

Suggested Manhole Diameter (mm) 1200

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.108	0.2	Kick-Flo®	0.078	0.2
Flush-Flo™	0.046	0.2	Mean Flow over Head Range	-	0.2

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	0.2	1.200	0.6	3.000	0.9	7.000	1.4
0.200	0.3	1.400	0.6	3.500	1.0	7.500	1.4
0.300	0.3	1.600	0.6	4.000	1.0	8.000	1.4
0.400	0.3	1.800	0.7	4.500	1.1	8.500	1.5
0.500	0.4	2.000	0.7	5.000	1.1	9.000	1.5
0.600	0.4	2.200	0.8	5.500	1.2	9.500	1.6
0.800	0.5	2.400	0.8	6.000	1.3		
1.000	0.5	2.600	0.8	6.500	1.3		

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<u>Cascade Summary of Results for 220830 GreenBlue Roof (G4).SRCX</u>					
Upstream Structures	Outflow To		Overflow To		
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	16.711	0.011	0.0	0.5	Flood Risk
30 min Summer	16.715	0.015	0.1	0.6	Flood Risk
60 min Summer	16.718	0.018	0.1	0.7	Flood Risk
120 min Summer	16.720	0.020	0.1	0.8	Flood Risk
180 min Summer	16.721	0.021	0.1	0.9	Flood Risk
240 min Summer	16.721	0.021	0.1	0.9	Flood Risk
360 min Summer	16.721	0.021	0.1	0.9	Flood Risk
480 min Summer	16.721	0.021	0.1	0.9	Flood Risk
600 min Summer	16.721	0.021	0.1	0.9	Flood Risk
720 min Summer	16.720	0.020	0.1	0.8	Flood Risk
960 min Summer	16.719	0.019	0.1	0.8	Flood Risk
1440 min Summer	16.718	0.018	0.1	0.7	Flood Risk
2160 min Summer	16.716	0.016	0.1	0.7	Flood Risk
2880 min Summer	16.715	0.015	0.1	0.6	Flood Risk
4320 min Summer	16.713	0.013	0.0	0.5	Flood Risk
5760 min Summer	16.712	0.012	0.0	0.5	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	0.4	18	
30 min Summer	58.468	0.0	0.6	32	
60 min Summer	38.020	0.0	0.8	60	
120 min Summer	23.997	0.0	1.0	90	
180 min Summer	18.178	0.0	1.2	122	
240 min Summer	14.896	0.0	1.3	156	
360 min Summer	11.219	0.0	1.5	224	
480 min Summer	9.163	0.0	1.6	290	
600 min Summer	7.826	0.0	1.7	354	
720 min Summer	6.878	0.0	1.8	418	
960 min Summer	5.608	0.0	2.0	540	
1440 min Summer	4.204	0.0	2.2	792	
2160 min Summer	3.150	0.0	2.5	1152	
2880 min Summer	2.565	0.0	2.7	1504	
4320 min Summer	1.917	0.0	3.1	2248	
5760 min Summer	1.558	0.0	3.3	2944	
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<u>Cascade Summary of Results for 220830 GreenBlue Roof (G4).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Summer	16.711	0.011	0.0	0.4	Flood Risk
8640 min Summer	16.710	0.010	0.0	0.4	Flood Risk
10080 min Summer	16.710	0.010	0.0	0.4	Flood Risk
15 min Winter	16.712	0.012	0.0	0.5	Flood Risk
30 min Winter	16.716	0.016	0.1	0.7	Flood Risk
60 min Winter	16.720	0.020	0.1	0.8	Flood Risk
120 min Winter	16.722	0.022	0.1	0.9	Flood Risk
180 min Winter	16.722	0.022	0.1	0.9	Flood Risk
240 min Winter	16.723	0.023	0.1	0.9	Flood Risk
360 min Winter	16.722	0.022	0.1	0.9	Flood Risk
480 min Winter	16.722	0.022	0.1	0.9	Flood Risk
600 min Winter	16.721	0.021	0.1	0.9	Flood Risk
720 min Winter	16.720	0.020	0.1	0.8	Flood Risk
960 min Winter	16.719	0.019	0.1	0.8	Flood Risk
1440 min Winter	16.717	0.017	0.1	0.7	Flood Risk
2160 min Winter	16.714	0.014	0.0	0.6	Flood Risk
2880 min Winter	16.713	0.013	0.0	0.5	Flood Risk
4320 min Winter	16.711	0.011	0.0	0.5	Flood Risk
5760 min Winter	16.710	0.010	0.0	0.4	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Summer	1.326	0.0	3.6	3680	
8640 min Summer	1.162	0.0	3.7	4408	
10080 min Summer	1.039	0.0	3.9	5136	
15 min Winter	84.523	0.0	0.5	18	
30 min Winter	58.468	0.0	0.7	32	
60 min Winter	38.020	0.0	0.9	58	
120 min Winter	23.997	0.0	1.2	94	
180 min Winter	18.178	0.0	1.3	132	
240 min Winter	14.896	0.0	1.5	168	
360 min Winter	11.219	0.0	1.7	238	
480 min Winter	9.163	0.0	1.8	304	
600 min Winter	7.826	0.0	1.9	372	
720 min Winter	6.878	0.0	2.0	440	
960 min Winter	5.608	0.0	2.2	560	
1440 min Winter	4.204	0.0	2.5	808	
2160 min Winter	3.150	0.0	2.8	1192	
2880 min Winter	2.565	0.0	3.1	1524	
4320 min Winter	1.917	0.0	3.4	2288	
5760 min Winter	1.558	0.0	3.7	2960	
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<u>Cascade Summary of Results for 220830 GreenBlue Roof (G4).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Winter	16.709	0.009	0.0	0.4	Flood Risk
8640 min Winter	16.709	0.009	0.0	0.4	Flood Risk
10080 min Winter	16.708	0.008	0.0	0.3	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Winter	1.326	0.0	4.0	3752	
8640 min Winter	1.162	0.0	4.2	4208	
10080 min Winter	1.039	0.0	4.4	4928	
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<u>Cascade Summary of Results for 220830 GreenBlue Roof (G5).SRCX</u>					
Upstream Structures		Outflow To		Overflow To	
(None) 220830 Cellular Tank.SRCX 220830 Cellular Tank.SRCX					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
15 min Summer	16.714	0.014	0.0	0.6	Flood Risk
30 min Summer	16.718	0.018	0.1	0.8	Flood Risk
60 min Summer	16.722	0.022	0.1	1.0	Flood Risk
120 min Summer	16.725	0.025	0.1	1.1	Flood Risk
180 min Summer	16.726	0.026	0.1	1.1	Flood Risk
240 min Summer	16.726	0.026	0.1	1.2	Flood Risk
360 min Summer	16.726	0.026	0.1	1.2	Flood Risk
480 min Summer	16.726	0.026	0.1	1.2	Flood Risk
600 min Summer	16.726	0.026	0.1	1.1	Flood Risk
720 min Summer	16.725	0.025	0.1	1.1	Flood Risk
960 min Summer	16.724	0.024	0.1	1.0	Flood Risk
1440 min Summer	16.721	0.021	0.1	0.9	Flood Risk
2160 min Summer	16.719	0.019	0.1	0.8	Flood Risk
2880 min Summer	16.717	0.017	0.1	0.8	Flood Risk
4320 min Summer	16.715	0.015	0.1	0.7	Flood Risk
5760 min Summer	16.714	0.014	0.0	0.6	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
15 min Summer	84.523	0.0	0.6	18	
30 min Summer	58.468	0.0	0.8	32	
60 min Summer	38.020	0.0	1.1	60	
120 min Summer	23.997	0.0	1.4	90	
180 min Summer	18.178	0.0	1.6	124	
240 min Summer	14.896	0.0	1.7	156	
360 min Summer	11.219	0.0	2.0	224	
480 min Summer	9.163	0.0	2.2	290	
600 min Summer	7.826	0.0	2.3	354	
720 min Summer	6.878	0.0	2.4	418	
960 min Summer	5.608	0.0	2.6	540	
1440 min Summer	4.204	0.0	3.0	792	
2160 min Summer	3.150	0.0	3.4	1148	
2880 min Summer	2.565	0.0	3.7	1528	
4320 min Summer	1.917	0.0	4.1	2248	
5760 min Summer	1.558	0.0	4.5	2952	
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<u>Cascade Summary of Results for 220830 GreenBlue Roof (G5).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Summer	16.713	0.013	0.0	0.6	Flood Risk
8640 min Summer	16.712	0.012	0.0	0.5	Flood Risk
10080 min Summer	16.711	0.011	0.0	0.5	Flood Risk
15 min Winter	16.716	0.016	0.1	0.7	Flood Risk
30 min Winter	16.721	0.021	0.1	0.9	Flood Risk
60 min Winter	16.725	0.025	0.1	1.1	Flood Risk
120 min Winter	16.727	0.027	0.1	1.2	Flood Risk
180 min Winter	16.728	0.028	0.1	1.2	Flood Risk
240 min Winter	16.728	0.028	0.1	1.3	Flood Risk
360 min Winter	16.728	0.028	0.1	1.2	Flood Risk
480 min Winter	16.727	0.027	0.1	1.2	Flood Risk
600 min Winter	16.726	0.026	0.1	1.1	Flood Risk
720 min Winter	16.725	0.025	0.1	1.1	Flood Risk
960 min Winter	16.723	0.023	0.1	1.0	Flood Risk
1440 min Winter	16.720	0.020	0.1	0.9	Flood Risk
2160 min Winter	16.717	0.017	0.1	0.8	Flood Risk
2880 min Winter	16.715	0.015	0.1	0.7	Flood Risk
4320 min Winter	16.713	0.013	0.0	0.6	Flood Risk
5760 min Winter	16.712	0.012	0.0	0.5	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Summer	1.326	0.0	4.8	3680	
8640 min Summer	1.162	0.0	5.0	4416	
10080 min Summer	1.039	0.0	5.2	5144	
15 min Winter	84.523	0.0	0.6	18	
30 min Winter	58.468	0.0	0.9	32	
60 min Winter	38.020	0.0	1.2	58	
120 min Winter	23.997	0.0	1.6	94	
180 min Winter	18.178	0.0	1.8	132	
240 min Winter	14.896	0.0	2.0	168	
360 min Winter	11.219	0.0	2.2	240	
480 min Winter	9.163	0.0	2.4	308	
600 min Winter	7.826	0.0	2.6	374	
720 min Winter	6.878	0.0	2.7	436	
960 min Winter	5.608	0.0	3.0	560	
1440 min Winter	4.204	0.0	3.3	810	
2160 min Winter	3.150	0.0	3.8	1172	
2880 min Winter	2.565	0.0	4.1	1560	
4320 min Winter	1.917	0.0	4.6	2252	
5760 min Winter	1.558	0.0	5.0	3056	
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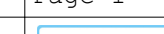
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<u>Cascade Summary of Results for 220830 GreenBlue Roof (G5).SRCX</u>					
Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m³)	Status
7200 min Winter	16.711	0.011	0.0	0.5	Flood Risk
8640 min Winter	16.710	0.010	0.0	0.4	Flood Risk
10080 min Winter	16.710	0.010	0.0	0.4	Flood Risk
Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)	
7200 min Winter	1.326	0.0	5.3	3816	
8640 min Winter	1.162	0.0	5.6	4256	
10080 min Winter	1.039	0.0	5.8	5000	
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<u>Cascade Model Details for 220830 GreenBlue Roof (G5).SRCX</u> Storage is Online Cover Level (m) 16.900 <u>Tank or Pond Structure</u> Invert Level (m) 16.700 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>44.1</td><td>0.108</td><td>44.1</td><td>0.109</td><td>0.0</td></tr></table> <u>Hydro-Brake® Optimum Outflow Control</u> <table><tr><td>Unit Reference</td><td>MD-SHE-0028-2000-0109-2000</td></tr><tr><td>Design Head (m)</td><td>0.109</td></tr><tr><td>Design Flow (l/s)</td><td>0.2</td></tr><tr><td>Flush-Flo™</td><td>Calculated</td></tr><tr><td>Objective</td><td>Minimise upstream storage</td></tr><tr><td>Application</td><td>Surface</td></tr><tr><td>Sump Available</td><td>Yes</td></tr><tr><td>Diameter (mm)</td><td>28</td></tr><tr><td>Invert Level (m)</td><td>16.700</td></tr><tr><td>Minimum Outlet Pipe Diameter (mm)</td><td>75</td></tr><tr><td>Suggested Manhole Diameter (mm)</td><td>1200</td></tr></table> <table><tr><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th><th>Control Points</th><th>Head (m)</th><th>Flow (l/s)</th></tr><tr><td>Design Point (Calculated)</td><td>0.109</td><td>0.2</td><td>Kick-Flo®</td><td>0.079</td><td>0.2</td></tr><tr><td>Flush-Flo™</td><td>0.046</td><td>0.2</td><td>Mean Flow over Head Range</td><td>-</td><td>0.2</td></tr></table> The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated <table><tr><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th><th>Depth (m)</th><th>Flow (l/s)</th></tr><tr><td>0.100</td><td>0.2</td><td>1.200</td><td>0.6</td><td>3.000</td><td>0.9</td><td>7.000</td><td>1.3</td></tr><tr><td>0.200</td><td>0.3</td><td>1.400</td><td>0.6</td><td>3.500</td><td>1.0</td><td>7.500</td><td>1.4</td></tr><tr><td>0.300</td><td>0.3</td><td>1.600</td><td>0.6</td><td>4.000</td><td>1.0</td><td>8.000</td><td>1.4</td></tr><tr><td>0.400</td><td>0.3</td><td>1.800</td><td>0.7</td><td>4.500</td><td>1.1</td><td>8.500</td><td>1.5</td></tr><tr><td>0.500</td><td>0.4</td><td>2.000</td><td>0.7</td><td>5.000</td><td>1.1</td><td>9.000</td><td>1.5</td></tr><tr><td>0.600</td><td>0.4</td><td>2.200</td><td>0.8</td><td>5.500</td><td>1.2</td><td>9.500</td><td>1.6</td></tr><tr><td>0.800</td><td>0.5</td><td>2.400</td><td>0.8</td><td>6.000</td><td>1.2</td><td></td><td></td></tr><tr><td>1.000</td><td>0.5</td><td>2.600</td><td>0.8</td><td>6.500</td><td>1.3</td><td></td><td></td></tr></table>						Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	0.000	44.1	0.108	44.1	0.109	0.0	Unit Reference	MD-SHE-0028-2000-0109-2000	Design Head (m)	0.109	Design Flow (l/s)	0.2	Flush-Flo™	Calculated	Objective	Minimise upstream storage	Application	Surface	Sump Available	Yes	Diameter (mm)	28	Invert Level (m)	16.700	Minimum Outlet Pipe Diameter (mm)	75	Suggested Manhole Diameter (mm)	1200	Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	0.109	0.2	Kick-Flo®	0.079	0.2	Flush-Flo™	0.046	0.2	Mean Flow over Head Range	-	0.2	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	0.2	1.200	0.6	3.000	0.9	7.000	1.3	0.200	0.3	1.400	0.6	3.500	1.0	7.500	1.4	0.300	0.3	1.600	0.6	4.000	1.0	8.000	1.4	0.400	0.3	1.800	0.7	4.500	1.1	8.500	1.5	0.500	0.4	2.000	0.7	5.000	1.1	9.000	1.5	0.600	0.4	2.200	0.8	5.500	1.2	9.500	1.6	0.800	0.5	2.400	0.8	6.000	1.2			1.000	0.5	2.600	0.8	6.500	1.3		
Depth (m)	Area (m ²)	Depth (m)	Area (m ²)	Depth (m)	Area (m ²)																																																																																																																												
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Appendix C : Water Network Calculations [Micro Drainage Network Module]

Internal Site Surface Water Drainage Network Calculations

DBFL Consulting Engineers		Page 1
Ormond House Upper Ormond Quay Dublin 7	158A Richmond Road Internal SW Drainage	
Date 24/01/2023 10:40 File 230120 SW Network Calcul...	Designed by DCB Checked by	
Innovyze	Network 2020.1	

STORM SEWER DESIGN by the Modified Rational Method

Pipe Sizes STANDARD Manhole Sizes STANDARD

Designed with Level Soffits

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.000	4-8	0.107	8-12	0.013

Total Pipe Volume (m³) = 21.561

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section	Type	Auto Design
S1.000	47.794	0.318	150.3	0.024	4.00	0.0	0.600	o	225	Pipe/Conduit		
S1.001	11.973	0.110	108.8	0.024	0.00	0.0	0.600	o	225	Pipe/Conduit		
S2.000	20.085	0.134	149.9	0.024	4.00	0.0	0.600	o	225	Pipe/Conduit		

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
S1.000	50.00	4.75	3.600	0.024	0.0	0.0	0.0	1.06	42.3	3.2
S1.001	50.00	4.91	3.282	0.048	0.0	0.0	0.0	1.25	49.8	6.5
S2.000	50.00	4.31	3.379	0.024	0.0	0.0	0.0	1.07	42.4	3.2

DBFL Consulting Engineers		Page 5																																																																								
Ormond House Upper Ormond Quay Dublin 7	158A Richmond Road Internal SW Drainage																																																																									
Date 24/01/2023 10:40 File 230120 SW Network Calcul...	Designed by DCB Checked by																																																																									
Innovyze Network 2020.1																																																																										
Online Controls for Storm																																																																										
Hydro-Brake® Optimum Manhole: S6, DS/PN: S1.004, Volume (m³): 1.1																																																																										
Unit Reference MD-SHE-0070-2000-0776-2000 Design Head (m) 0.776 Design Flow (l/s) 2.0 Flush-Flo™ Calculated Objective Minimise upstream storage Application Surface Sump Available Yes Diameter (mm) 70 Invert Level (m) 3.156 Minimum Outlet Pipe Diameter (mm) 100 Suggested Manhole Diameter (mm) 1200																																																																										
<table><tr><td>Control Points</td><td>Head (m)</td><td>Flow (l/s)</td><td>Control Points</td><td>Head (m)</td><td>Flow (l/s)</td></tr><tr><td>Design Point (Calculated)</td><td>0.776</td><td>2.0</td><td>Kick-Flo®</td><td>0.493</td><td>1.6</td></tr><tr><td>Flush-Flo™</td><td>0.232</td><td>2.0</td><td>Mean Flow over Head Range</td><td>-</td><td>1.7</td></tr></table>			Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	0.776	2.0	Kick-Flo®	0.493	1.6	Flush-Flo™	0.232	2.0	Mean Flow over Head Range	-	1.7																																																						
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DBFL Consulting Engineers			Page 6		
Ormond House Upper Ormond Quay Dublin 7		158A Richmond Road Internal SW Drainage			
Date 24/01/2023 10:40 File 230120 SW Network Calcul...		Designed by DCB Checked by			
Innovyze		Network 2020.1			
<u>Storage Structures for Storm</u>					
<u>Porous Car Park Manhole: S2, DS/PN: S2.000</u>					
Infiltration Coefficient Base (m/hr) 0.16900					

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Ormond House Upper Ormond Quay Dublin 7		158A Richmond Road Internal SW Drainage	
Date 24/01/2023 10:40 File 230120 SW Network Calcul...		Designed by DCB Checked by	
Innovyze		Network 2020.1	

Summary of Critical Results by Maximum Level (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000	Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0	MADD Factor * 10m ³ /ha Storage 2.000
Hot Start Level (mm) 0	Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500	Flow per Person per Day (l/per/day) 0.000
Foul Sewage per hectare (l/s) 0.000	

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 2
 Number of Online Controls 1 Number of Storage Structures 2 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model	FSR	Ratio R 0.278
Region	Scotland and Ireland Cv (Summer) 0.750	
M5-60 (mm)	16.100 Cv (Winter) 0.840	

Margin for Flood Risk Warning (mm) 300.0 DVD Status OFF
 Analysis Timestep Fine Inertia Status OFF
 DTS Status ON

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 10080
 Return Period(s) (years) 1, 30, 100
 Climate Change (%) 20, 20, 20

PN	US/MH Name	Event	Duration (mins)	US/CL (m)	Water	Surcharged	Flooded
					Level (m)	Depth (m)	Volume (m ³)
S1.000	S1	10080 minute 100 year Winter I+20%	10080	5.150	3.615	-0.210	0.000
S1.001	S2	15 minute 100 year Winter I+20%	15	4.700	3.361	-0.146	0.000
S2.000	S2	10080 minute 100 year Winter I+20%	10080	5.000	3.395	-0.209	0.000
S2.001	S3	15 minute 100 year Winter I+20%	15	5.000	3.351	-0.119	0.000
S1.002	S3	15 minute 100 year Winter I+20%	15	4.700	3.332	-0.500	0.000
S1.003	S6	180 minute 100 year Winter I+20%	180	4.179	3.288	-0.109	0.000
S1.004	S6	480 minute 100 year Winter I+20%	480	4.100	3.355	-0.026	0.000
S1.005	S4	180 minute 100 year Winter I+20%	180	4.100	2.987	-0.186	0.000
S1.006	S5	180 minute 100 year Winter I+20%	180	4.300	2.956	-0.190	0.000
S1.007	S6	180 minute 100 year Winter I+20%	180	4.700	2.830	-0.187	0.000

Pipe

PN	US/MH Name	Flow / Cap.	Overflow (l/s)	Maximum Vol (m ³)	Flow (l/s)	Status
S1.000	S1	0.01		0.012	0.5	OK

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Richmond Road Works Surface Water Drainage Network Calculations

DBFL Consulting Engineers		Page 1
Ormond House Upper Ormond Quay Dublin 7	158A Richmond Road Richmond Road Works Drainage	
Date 24/01/2023 10:45 File 20230109 Richmond Road B...	Designed by DCB Checked by	
Innovyze	Network 2020.1	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - Scotland and Ireland

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	16.100	Add Flow / Climate Change (%)	0
Ratio R	0.278	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.169	4-8	0.101

Total Area Contributing (ha) = 0.270

Total Pipe Volume (m³) = 10.277

Network Design Table for Storm

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	46.231	0.191	242.0	0.067	4.00	0.0	0.600	o	300	Pipe/Conduit	
1.001	70.228	0.234	300.1	0.067	0.00	0.0	0.600	o	300	Pipe/Conduit	
1.002	26.275	0.088	298.6	0.067	0.00	0.0	0.600	o	300	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	4.77	3.300	0.067	0.0	0.0	0.0	1.01	71.1	9.1
1.001	50.00	6.06	3.109	0.134	0.0	0.0	0.0	0.90	63.8	18.1
1.002	50.00	6.55	2.875	0.201	0.0	0.0	0.0	0.90	63.9	27.2

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Ormond House Upper Ormond Quay Dublin 7	158A Richmond Road Richmond Road Works Drainage	
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Innovyze	Network 2020.1	

PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	o	300	1	4.986	3.300	1.386	Open Manhole	1200
1.001	o	300	2	4.564	3.109	1.155	Open Manhole	1200
1.002	o	300	3	4.341	2.875	1.166	Open Manhole	1200
1.003	o	150	4	4.706	2.787	1.769	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	46.231	242.0	2	4.564	3.109	1.155	Open Manhole	1200
1.001	70.228	300.1	3	4.341	2.875	1.166	Open Manhole	1200
1.002	26.275	298.6	4	4.706	2.787	1.619	Open Manhole	1200
1.003	10.610	279.2		4.529	2.749	1.630	Open Manhole	0

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Ormond House Upper Ormond Quay Dublin 7	158A Richmond Road Richmond Road Works Drainage	
Date 24/01/2023 10:45 File 20230109 Richmond Road B...	Designed by DCB Checked by	
Innovyze	Network 2020.1	

Area Summary for Storm

Pipe Number	PIMP Type	PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	-	-	100	0.067	0.067	0.067
1.001	-	-	100	0.067	0.067	0.067
1.002	-	-	100	0.067	0.067	0.067
1.003	-	-	100	0.069	0.069	0.069
				Total	Total	Total
				0.270	0.270	0.270

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
1.003		4.529	2.749	0.000	0	0

Simulation Criteria for Storm

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0
 Number of Online Controls 0 Number of Storage Structures 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	0.750
Region	Scotland and Ireland	Cv (Winter)	0.840
M5-60 (mm)	17.200	Storm Duration (mins)	30
Ratio R	0.302		

©1982-2020 Innovyze

DBFL Consulting Engineers		Page 5
Ormond House Upper Ormond Quay Dublin 7	158A Richmond Road Richmond Road Works Drainage	
Date 24/01/2023 10:45	Designed by DCB	
File 20230109 Richmond Road B...	Checked by	
Innovyze	Network 2020.1	

Summary of Critical Results by Maximum Level (Rank 1) for Storm

Areal Reduction Factor	1.000	Additional Flow - % of Total Flow	0.000
Hot Start (mins)	0	MADD Factor * 10m³/ha Storage	2.000
Hot Start Level (mm)	0	Inlet Coeffiecient	0.800
Manhole Headloss Coeff (Global)	0.500	Flow per Person per Day (l/per/day)	0.000
Foul Sewage per hectare (l/s)	0.000		

Synthetic Rainfall Details

PN	US/MH Name	Overflow (l/s)	Maximum Vol (m³)	Pipe Flow (l/s)	Status
1.000	1		1.128	22.3	SURCHARGED
1.001	2		4.504	27.6	FLOOD RISK
1.002	3		6.408	41.0	FLOOD RISK
1.003	4		3.348	55.0	SURCHARGED



Appendix D : Foul Water Network Calculations [Micro Drainage]



Appendix E : Irish Water Confirmation of Feasibility

CONFIRMATION OF FEASIBILITY

Dieter Bester

DBFL
Ormond House
Ormond Quay Upper
Dublin
Dublin
D07 W704

Uisce Éireann
Bosca OP448
Oifig Sheachadta na
Cathrach Theas
Cathair Chorcaí

Irish Water
PO Box 448,
South City
Delivery Office
Cork City.

www.water.ie

16 August 2022

Our Ref: CDS22005766 Pre-Connection Enquiry
158A Richmond Road, Drummcondra, Co. Dublin

Dear Applicant/Agent,

We have completed the review of the Pre-Connection Enquiry.

Irish Water has reviewed the pre-connection enquiry in relation to a Water & Wastewater connection for a Multi/Mixed Use Development of 150 unit(s) at 158A Richmond Road, Drummcondra, Co. Dublin, (the **Development**).

Based upon the details provided we can advise the following regarding connecting to the networks;

- **Water Connection** - Feasible without infrastructure upgrade by Irish Water
- **Wastewater Connection** - Feasible without infrastructure upgrade by Irish Water

This letter does not constitute an offer, in whole or in part, to provide a connection to any Irish Water infrastructure. Before the Development can be connected to our network(s) you must submit a connection application and be granted and sign a connection agreement with Irish Water.

As the network capacity changes constantly, this review is only valid at the time of its completion. As soon as planning permission has been granted for the Development, a completed connection application should be submitted. The connection application is available at www.water.ie/connections/get-connected/

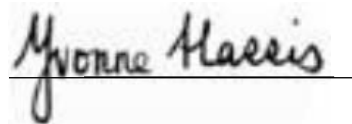
Where can you find more information?

- **Section A** - What is important to know?

This letter is issued to provide information about the current feasibility of the proposed connection(s) to Irish Water's network(s). This is not a connection offer and capacity in Irish Water's network(s) may only be secured by entering into a connection agreement with Irish Water.

For any further information, visit www.water.ie/connections, email newconnections@water.ie or contact 1800 278 278.

Yours sincerely,

A handwritten signature in black ink that reads "Yvonne Harris". The signature is written in a cursive style and is positioned above a horizontal line.

Yvonne Harris
Head of Customer Operations

Section A - What is important to know?

What is important to know?	Why is this important?
Do you need a contract to connect?	• Yes, a contract is required to connect. This letter does not constitute a contract or an offer in whole or in part to provide a connection to Irish Water's network(s). • Before the Development can connect to Irish Water's network(s), you must submit a connection application <u>and be granted and sign</u> a connection agreement with Irish Water.
When should I submit a Connection Application?	• A connection application should only be submitted after planning permission has been granted.
Where can I find information on connection charges?	• Irish Water connection charges can be found at: https://www.water.ie/connections/information/charges/
Who will carry out the connection work?	• All works to Irish Water's network(s), including works in the public space, must be carried out by Irish Water*. *Where a Developer has been granted specific permission and has been issued a connection offer for Self-Lay in the Public Road/Area, they may complete the relevant connection works
Fire flow Requirements	• The Confirmation of Feasibility does not extend to fire flow requirements for the Development. Fire flow requirements are a matter for the Developer to determine. • What to do? - Contact the relevant Local Fire Authority
Plan for disposal of storm water	• The Confirmation of Feasibility does not extend to the management or disposal of storm water or ground waters. • What to do? - Contact the relevant Local Authority to discuss the management or disposal of proposed storm water or ground water discharges.
Where do I find details of Irish Water's network(s)?	• Requests for maps showing Irish Water's network(s) can be submitted to: datarequests@water.ie

What are the design requirements for the connection(s)?	• The design and construction of the Water & Wastewater pipes and related infrastructure to be installed in this Development shall comply with <i>the Irish Water Connections and Developer Services Standard Details and Codes of Practice</i>, available at www.water.ie/connections
Trade Effluent Licensing	• Any person discharging trade effluent** to a sewer, must have a Trade Effluent Licence issued pursuant to section 16 of the Local Government (Water Pollution) Act, 1977 (as amended). • More information and an application form for a Trade Effluent License can be found at the following link: https://www.water.ie/business/trade-effluent/about/ **trade effluent is defined in the Local Government (Water Pollution) Act, 1977 (as amended)



Appendix F : Irish Water Statement of Design Acceptance

Dieter Bester
Ormond House
Ormond Quay Upper
Dublin, Dublin D07 W704

Uisce Éireann
Bosca OP 448
Oifig Sheachadta na
Cathrach Theas
Cathair Chorcaí

Irish Water
PO Box 448,
South City
Delivery Office,
Cork City.

www.water.ie

5 September 2022

**Re: Design Submission for 158A Richmond Road, Drummcondra, Co. Dublin (the “Development”)
(the “Design Submission”) / Connection Reference No: CDS22005766**

Dear Dieter Bester,

Many thanks for your recent Design Submission.

We have reviewed your proposal for the connection(s) at the Development. Based on the information provided, which included the documents outlined in Appendix A to this letter, Irish Water has no objection to your proposals.

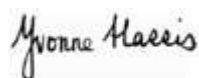
This letter does not constitute an offer, in whole or in part, to provide a connection to any Irish Water infrastructure. Before you can connect to our network you must sign a connection agreement with Irish Water. This can be applied for by completing the connection application form at www.water.ie/connections. Irish Water's current charges for water and wastewater connections are set out in the Water Charges Plan as approved by the Commission for Regulation of Utilities (CRU) (https://www.cru.ie/document_group/irish-waters-water-charges-plan-2018/).

You the Customer (including any designers/contractors or other related parties appointed by you) is entirely responsible for the design and construction of all water and/or wastewater infrastructure within the Development which is necessary to facilitate connection(s) from the boundary of the Development to Irish Water's network(s) (the “**Self-Lay Works**”), as reflected in your Design Submission. Acceptance of the Design Submission by Irish Water does not, in any way, render Irish Water liable for any elements of the design and/or construction of the Self-Lay Works.

If you have any further questions, please contact your Irish Water representative:

Name: Marina Byrne
Phone: 01 89 25991/ 087619321
Email: mzbyrne@water.ie

Yours sincerely,



Yvonne Harris
Head of Customer Operations

Appendix A

Document Title & Revision

- 210178-DBFL-CS-SP-DR-C-1300-C-1300
- 210178-DBFL-CS-SP-DR-C-3311 Longitudinal Sections Through Foul Water Sewers - Sheet 1

Additional Comments

The design submission will be subject to further technical review at connection application stage.

While Irish Water notes that the wastewater services infrastructure will remain private and not be vested, we have the following comments: It is recommended that the foul sewer should have 3 m clearance from the proposed building.

Irish Water cannot guarantee that its Network in any location will have the capacity to deliver a particular flow rate and associated residual pressure to meet the requirements of the relevant Fire Authority, see Section 1.17 of Water Code of Practice.

For further information, visit www.water.ie/connections

Notwithstanding any matters listed above, the Customer (including any appointed designers/contractors, etc.) is entirely responsible for the design and construction of the Self-Lay Works. Acceptance of the Design Submission by Irish Water will not, in any way, render Irish Water liable for any elements of the design and/or construction of the Self-Lay Works.



Appendix G : Stormbloc Brochure and Maintenance Procedures

Attenuate surface water and stormwater effectively even in challenging high-traffic urban environments.

Stormbloc® Optimum is a resilient geocellular storage system that provides underground storage and infiltration of urban runoff.

Lightweight materials combined with robust design make it easy to transport, quick to install and extremely durable, even beneath high-traffic areas such as roads, car parks and warehouse yards.

Stormbloc® Optimum is modular and easily customisable, giving you the freedom to configure storage for even the most challenging SuDS project.

Applications

- New and retrofit Sustainable Drainage (SuDS) schemes.
- Infiltration / soakaway schemes.
- Attenuation schemes.
- Highways and infrastructure projects.
- To increase swale / pond capacity.
- Car parks and Park & Ride schemes.
- Housing Developments.
- Schools and Public / Civil schemes.
- Aquifer re-charge.
- Storage for rainwater harvesting and re-use.

Inspection and Maintenance Access

Access shafts can be easily constructed during installation of the Stormbloc® Optimum system to enable inspection and maintenance.

The channels between the vertical struts allow access for CCTV inspection, maintenance and cleaning and ensures that the storage volume of the system isn't compromised by the build up of silts.



Benefits

Customise Your Storage

Length, width and depth of storage can be customised in order to meet even the most demanding of drainage environments. Two inspection channels enable easy inspection and maintenance.

Save Space On Site

Stormbloc® Optimum stacks into compact nests for transportation and storage, saving valuable space during transport and on site. The nested units can also enable more storage volume to be lifted into excavations at any one time, saving time and cost of lifting machinery. The units unstack simply for installation. With up to 75% space reduction, a double pallet of stacked, nested boxes delivers more than 14 m³ of stormwater storage, and a single lorry delivery can provide 345 m³.

Lightweight and Strong

Combining strength with a storage coefficient of 96%, Stormbloc® Optimum allows you to plan and design effective SuDS systems even in challenging urban environments with high traffic levels. The Stormbloc® Optimum can withstand heavy goods vehicle traffic with an overall load of up to 60 tons, and can be installed at base depths of up to 4 metres.

Technical information

	Stormbloc® Optimum Full Block	Stormbloc® Optimum Half Block
Material	Polypropylene	Polypropylene
Length / Width / Height	800 / 800 / 660 mm	800 / 800 / 360 mm
Nominal Block Volume	0.422 m ³	0.230 m ³
Nominal Storage Capacity per Unit	0.405 m ³	0.221 m ³
Constructed Block Weight	18.6 kg	13.7 kg
Porosity	96%	96%
Vertical Ultimate Compressive Strength	≥ 420 kN/m ²	≥ 420 kN/m ²
Horizontal Ultimate Compressive Strength	≥ 165 kN/m ²	≥ 225 kN/m ²



Stormbloc® Optimum inspection and maintenance channel.



Stormbloc® Optimum installation.

Learn more

To learn more about how Stormbloc® Optimum can help you to make better water management decisions, visit hydro-int.com, search **Stormbloc Optimum** online or contact us:

Americas

+1 (207) 756 6200
inquiries@hydro-int.com

Asia Pacific

+61 436 433 686
enquiries@hydro-int.com

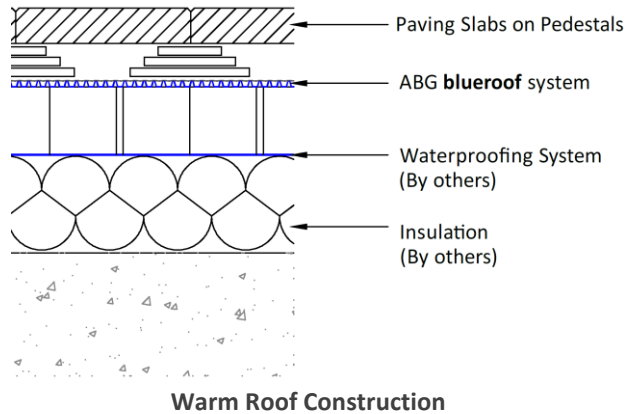
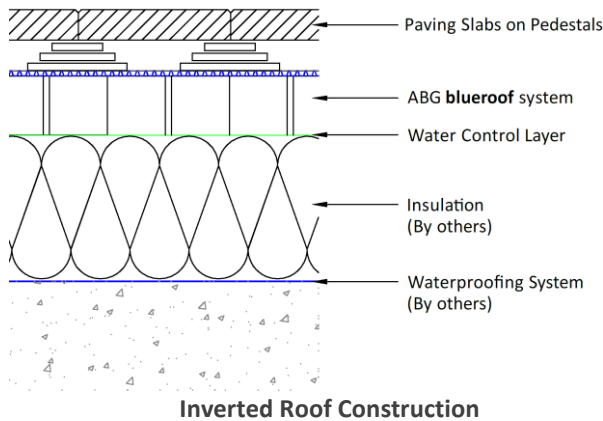
Europe & RoW

+44 (0)1275 878371
enquiries@hydro-int.com

Middle East

+971 506 026 400
enquiries@hydro-int.com

Appendix H : ABG Blue/Green Roof Brochure



ABG **bluroof** systems provide a constant drainage path, SuDS attenuation, filtration and controlled release of stormwater, combining all the key elements of a good SuDS design. The storage element of the system must be used in conjunction with the 'blue roof' restrictor chamber. These chambers are bespoke to each project in order to help achieve the project engineer's maximum discharge rates, and to suit the required build-up and final use of the podium/roof area. ABG's 'blue roofs' are generally used for zero falls, inverted/warm roof and podium applications, under a mix of hard and soft landscaped finishes. Other combinations of ABG **bluroof** systems and most surface finishes are available. Please refer to ABG's Technical team for project/system specific advice & 'blue roof' SuDS calculations.

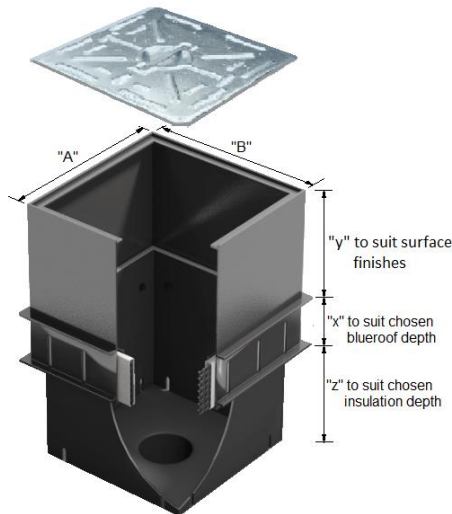
		ABG bluroof VF HD							
System Properties		58mm	80mm	108mm	130mm	158mm	180mm		
Thickness at 2kPa	(mm)	58	80	108	130	158	180	±10%	EN ISO 9863-1
Maximum saturated weight	(kg/m ²)	58	80	108	130	158	180	approx.	EN ISO 9864
Stormwater attenuation volume	(l/m ²)	50	65	97	113	145	160		
Growing medium recharge value	(l/m ²)	25	25	25	25	25	25	Per 100mm depth	
Drainable void space	%	86	81	90	87	92	89		
Resistance to weathering		Greater than 60% retained tensile strength						EN 12224	
Resistance to chemicals		Excellent						EN 14030	
Upper Filter/Separator Properties									
Pore size O ₉₀	(µm)	120				±30%		EN ISO 12956	
Breakthrough head	(mm)	0				nominal		BS 6906 Part 3	
CBR puncture resistance	(N)	1 600				-20%		EN ISO 12236	
Dynamic perforation cone drop	(mm)	32				+20%		EN ISO 13433	
Type and material	Non-woven needle-punched and heat-treated long staple fibre polypropylene Protector: Non-woven felt of polypropylene. Min wt. of 120g/m ²								
'Blue roof' system use & compatible surface finishes									
Suitable for ABG Load Class 2 (Pedestrians, cycles and light vehicles, MUGAs, medium sized plant installations). Landscaped, paved or permeable resin-bound gravel finishes.									

Notes

- The values given are indicative and correspond to nominal results obtained in our laboratories and testing institutes. In line with our policy of continuous improvement the right is reserved to make changes without notice at any time.
- Any additional installations such as plant/services, PV panels, paved areas or additional vehicular/traffic access, must be discussed with ABG prior to their installation/use.
- Final determination of the suitability of any information is the sole responsibility of the user. ABG will be pleased to discuss the use of this or any other product but responsibility for selection of a material and its application in any specific project remains with the user.
- Can be used in conjunction with rainwater harvesting & grey water recycling systems. Any petrochemical pollution waste discharged from the system to be treated by others.

bluroof Restrictor Chamber

ABG **bluroof** Restrictor Chambers are purpose designed and patented as an integral part of ABG's **bluroof** system. Each chamber contains a restrictor valve and filter and when used in conjunction with our blue roof attenuation layers, controls the blue roof discharge rate in line with the planning consent of the site. Restrictor Chambers are available in 2 main types; a dense polypropylene chamber with galvanised lid for foot traffic only areas and a stainless steel chamber and lid for vehicular trafficked areas such as podiums or walkways (recessed lid also available). All chambers are accessible to allow for on-going maintenance. They are manufactured to order to specific heights as dictated by the chosen ABG **bluroof** system ("x"), surface finish ("y") and insulation depths ("z"). An alternative chamber profile is available when used with a parapet outlet.



Bespoke design specific heights



ABG Restrictor Chamber in place to insulation depth ready to receive the ABG bluroof system

Internal Restrictor Chamber – Polypropylene (maintenance foot traffic only) (illustrated above)

Materials	Upper chamber width (mm) ("A")	Upper chamber length (mm) ("B")	Application notes
Polypropylene chamber galvanised steel lid	361	361	Placed in open area of roof. Maintenance foot traffic only. No vehicular traffic. e.g. paved surface finish

Parapet Restrictor Chamber – Polypropylene (maintenance foot traffic only)(no ribs on one side to fit to parapet)

Materials	Upper chamber width (mm) ("A")	Upper chamber length (mm) ("B")	Application notes
Polypropylene chamber galvanised steel lid	361	341	Placed tight to parapet wall. Maintenance foot traffic only. No vehicular traffic. e.g. paved surface finish

Internal Restrictor Chamber – Steel (trafficked) (similar profile to illustration above)

Materials	Upper chamber width (mm) ("A")	Upper chamber length (mm) ("B")	Application notes
Stainless Steel chamber and lid	375	375	Placed in open area of roof. Pedestrian or vehicular traffic. e.g. podium deck

Parapet Restrictor Chamber – Steel (trafficked)(no ribs on one side to fit to parapet)

Materials	Upper chamber width (mm) ("A")	Upper chamber length (mm) ("B")	Application notes
Stainless Steel chamber and lid	375	350	Placed tight to parapet wall. Pedestrian or vehicular traffic. e.g. podium deck

bluroof Filter Geotextile Characteristics

Characteristic	Units	Value	Allowable variance	Standard
Pore size O_{90}	(μm)	115	$\pm 30\%$	EN ISO 12956
Water flow at 50mm head	$\text{l/m}^2.\text{s}$	105	-15%	EN ISO 11058
Type of material	Non woven needle-punched polypropylene geotextile			
Resistance to chemicals	Excellent			EN ISO 14030

Appendix I : Murphy Geospatial Utility Survey and Utility Survey Report

Utility Survey Report

Project Name – Leydons Richmond Road

Project Number – 43568

Client – Malkey Limited



Document Register

Rev	Date	Prepared by	Role	Checked by	Role	Revision Reason
00	28/10/2021	IP	CAD Technician	DS	Quality Manager	First issue

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1 Introduction

1.1 Terms of Reference

Location: **158A Richmond Rd, Ballybough, Dublin 3**

Client: **Malkey Limited**

Utility Survey Date start: **27/10/2021**

This report should be viewed with the following drawings: **MGS43568-U.dwg**

This document is the technical report for this investigation; it therefore supersedes any previous reports whether written or oral.

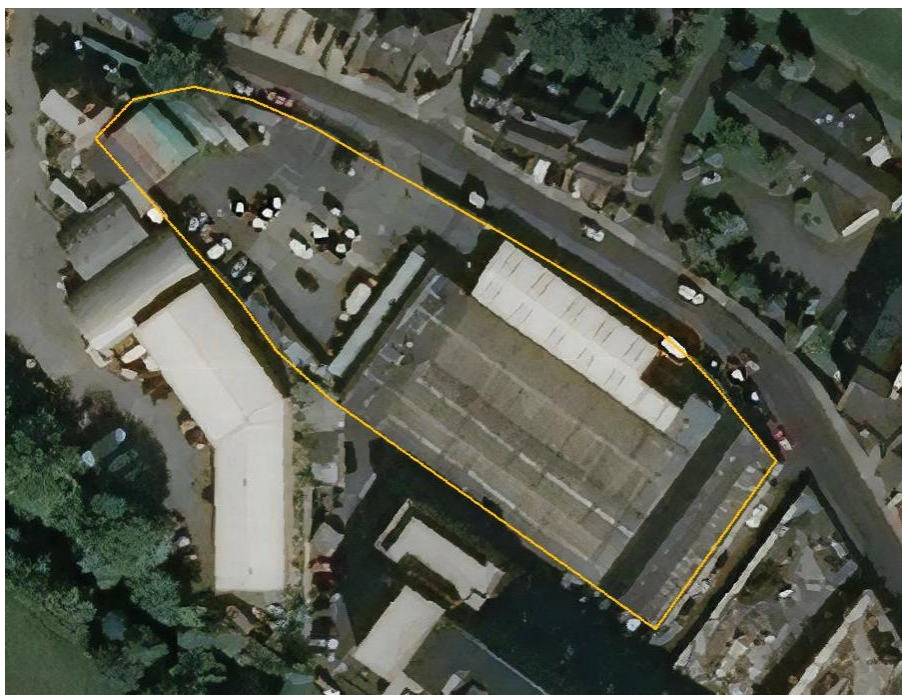
1.2 Background/Purpose of Investigation

Murphy Geospatial were requested to carry out a full GPR & utility survey on behalf of Malkey Limited. The intention of this survey is to detect, locate and record all existing Utilities and highlight any anomalies in the required area for upcoming works.

1.3 Objective of project

The objective of the survey was to locate the position and depth of all existing underground utilities using a combination of non-intrusive survey techniques. As the main investigative techniques used are largely non-destructive, the findings given in this report are based on indirect measurements and the interpretation of acoustic, electrical and electromagnetic signals. The findings represent the best professional opinions of the authors, based on our experience and the results of non-intrusive pipe location carried out elsewhere on similar materials and projects.

1.4 Site aerial view



1.5 Key Personnel

Project Manager: **Aidan Doherty**

Responsible for the management of the overall project

Senior Surveyor: **Luke Maguire**

Lead surveyor responsible for the site work

Safety Advisor: **Dermot Guiney**

Responsible for safety inductions (internal requirements only) and advising on safe working practices

Quality Manager: **Daniel Stempien**

Responsible for processing and quality assessment of data

1.6 Specifications and International Standards

All survey works were carried out in accordance with the following guidelines and standards:

- European GPR Association - Policy on the Use of GPR in Utility Detection
- American Society of Civil Engineers- Standard Guideline for the collection and depiction of existing subsurface utility data.
- Radio detection- abc & xyz of locating buried pipes and cables.
- PAS128: 2014 – Publicly Available Specification 128 2014

2 Survey Report

2.1 Survey Restrictions

There was limited or no access to some areas due to parked cars or containers/pallets located on the site – please refer to the drawing.



2.2 Field Data Survey

Survey was carried out in ITM OSGM15 coordinate system. All levels are related to Malin Head Datum. Survey results were overlaid on the topographical background.

2.3 Traffic Management

No Traffic Management was required for this project.

2.4 Methodology

2.4.1 Underground Utility & GPR Survey

Murphy Geospatial detect conductive services with the use of the Radio detection RD8100 receivers and transmitters which use very low electromagnetic frequencies to detect the services utilising the following methods:-

Direct Connection – This technique incorporates the use of a signal generator which is capable of generating sine waves at very low frequencies, typically 8 kHz or 33 kHz which can be applied to a metallic service. The service acts like an aerial and conducts the transmitted signal, which can then be detected on the surface using the receiver.

This is the most accurate method of locating a buried service and is applied in the first instance where access to pipes and cables is possible.

Signal Clamp – The signal clamp will be used to trace buried LV and HV cables. The signal will be applied via a clamp which is placed around the cable at a point the service enters or exits the ground.

Induction – Where a direct signal cannot be applied, the transmitter is used to radiate an indirect signal actively. The transmitter has a built-in aerial, which is capable of transmitting an electromagnetic field into the ground which conducts along the pipe or cable and can be detected on the surface using a receiver.

Passive – In Passive mode, the receiver is used without the transmitter to detect signals, which are generated by power cables or from distant radio transmitters, which constantly induce a signal into metallic services.

This method should only be used once both Direct Connection and Induction methods have been exhausted.

2.4.2 GPR Methodology:

A number of different GPR grids were set out over the site. Data field files were collected with a multi frequency array antenna system to give maximum depth penetration whilst maintaining a high resolution at both shallow and deep depths. Full calibration was carried out at the start scan with constant quality monitoring during acquisition and frequent recalibration checks were carried out where necessary.

Depth readings from GPR rely on multiplying the measured two-way travel time by the velocity of the radio signals passing through the materials under investigation. As the surface and subsurface of the site changes, frequent recalibration of the subsurface velocities results in an accurate calculation of depths and thicknesses of located features relative to the surface.

Post site processing then took place in the office, using specialised software, GPR Slice. A number of processing stages were involved, including start time correction, amplitude gain adjustments, Gaussian filtering, dynamic correction and noise removal. Once the raw data was processed individual targets were identified on each survey line and linear features mapped out over the survey areas. These GPR results are then incorporated into AutoCAD for final processing.

2.5 Equipment Used

RD4000/8000, Dual-Frequency GPR Radar System, Sonde, Copper Flex, Trimble S3 Total Station/R6 GPS Trimble

2.6 Surveyors Involved

Luke Maguire, Adam Maguire

2.7 Works Programme

Site Works Commenced in October 2021

Delivery of drawings – 28/10/2021

2.8 Software Used for Processing

AutoCAD Civil 3D 2019

AutoCAD 2019

GPR Slice V7.0

GREED HD 3D 01.02.00

GeoPal.

2.9 Quality Assurance Site Procedures

Equipment used was calibrated and tested in line with manufacturer guidelines.

Calibration certificates can be provided on request. Distance & angle checks were carried out on site regularly.

2.10 Findings

2.10.1 Drainage

Comments	Quality Level QL- A,B,C,D	Methodology M1 - M4
No evidence of foul/combined sewer network was identified within survey area. Storm water network was identified in manholes 3, 4, 6, 5, 2 and 1 running from manholes around the buildings towards manhole 1 where storm water is discharged to manhole offsite – please refer to the drawing. Please note that due to the siltage or blockage in the pipes, it was not possible to verify all drainage connections on site – please refer to the drawing. Drainage service records drawing doesn't show any connections within survey area.	B2P	M2P

2.10.2 Water Mains/Fire Mains

Comments	Quality Level QL- A,B,C,D	Methodology M1 - M4
Due to possible non-metallic nature, no signal was detected from water valve located offsite which is possible the access valve for the main building connection. Water main pipe were identified in manhole 1 and then these pipes are exposed on the nearby wall. Service records drawing shows only a main water pipe entering the site in the North-East section of the site. Water pipes which are shown on records drawings but which couldn't be located and verified on site were marked with 'records' note and it is recommended to treat their location as indicative only.	B2P (located) B4 (records)	M2P

2.10.3 Electricity, HV, LV, Street lighting, Traffic

Comments	Quality Level QL- A,B,C,D	Methodology M1 - M4
No evidence of Electrical cable was found within survey area. ESB records shows only electrical connections from offsite to EBOX located offsite. ESB connections which were shown on records but couldn't be located and verified don site were marked with 'records' note and it is recommended to treat their location as indicative only. Public lighting connections to the lamp poles were identified on site. Two lamp posts were reported as being disused – pleas refer to the drawing.	B4	M2P

2.10.4 Eir, Enet, UPC (Virgin), BT and other Comms

Comments	Quality Level QL- A,B,C,D	Methodology M1 - M4
No evidence of Enet, BT, Eir, UPC was identified on site. There is no Enet, BT, Eir, UPC in this area according to records data.	N/A	M2P

2.10.5 Gas, Oil & Fuel mains

Comments	Quality Level QL- A,B,C,D	Methodology M1 - M4
Due to non-metallic nature no signal was detected of gas pipe which is present within survey area according to the GNI records drawings. GPR results were also not fully conclusive for this gas pipe and only some sections of the gas network were identified on site – please refer to the drawing. Sections which are shown on records drawings but which couldn't be located and verified on site were marked with 'records' note and it is recommended to treat their location as indicative only.	B2P (located) B4 (records)	M2P

2.10.6 GPR data conclusion

Comments
Generally, the depth of investigation from GPR does not exceed 1.5 metres in this area. As well as all the confirmed utility services which have been identified, there are unidentified features shown as GPR Anomalies. These features may be the result of services which are running through the sites, abandoned services, natural geological features or land drains amongst other things.

2.11 Manhole and pit schedules

Each manhole/inspection cover within the survey area was opened and the contents documented. These measurements are recorded on a digital manhole description sheet using Geopal applications. The manholes were individually numbered. All depths recorded inside the chamber were by disto, measuring tape or levelling staff. Details included:

- Cover Levels
- Invert levels
- Service Type
- Service Material
- Pipe sizes
- Chamber dimensions
- Direction of flow
- Photographs

After completing manhole investigation each manhole sheets was exported to Excel format and submitted together with final drawing and GPR report as a part of final deliverables.

2.12 Recommendations

Services which are shown on service records drawings, but which couldn't be located and verified on site and services which couldn't be traced due to no signal being obtained or signal being lost during the trace will require further investigation.

It is recommended to carry out slit trenching investigation in this area which would allow identifying location and depths of these untraceable services.

Drainage lines which couldn't be traced will require further investigation. It is recommended to clean (jet wash) the pipes and carry out CCTV investigation works which would allow identifying connection points for these pipes.

It is recommended to clear the area from any obstacles and re-check those cleared areas for any potential evidence of the manholes or services in those currently obstructed sections of the site.

3 Pas Detection Methods and Quality Level Tables

	Survey type (Establish with client prior to survey)	Quality level (Practitioner to determine post survey)	Post-Processing	Location Accuracy		Supporting Data
				Horizontal ¹⁾	Vertical ²⁾	
D	Desktop utility records search	QL-D	-	Undefined	Undefined	-
C	Site reconnaissance	QL-C	-	Undefined	Undefined	A segment of utility whose location is demonstrated by visual reference to street furniture, topographical features or evidence of previous street works (reinstatement scar).
B	Detection	QL-B4	-	Undefined	Undefined	A utility segment which is suspected to exist but has not been detected and is therefore shown as an assumed route.
		QL-B3	No	±500 mm	Undefined (No reliable depth measurement possible)	Horizontal location only of the utility detected by one of the geophysical techniques used
		QL-B3P	Yes		±40% of detected depth	Horizontal and vertical location of the utility detected by one of the geophysical techniques used. ⁴⁾
		QL-B2	No	±250 mm or ±40% of detected depth whichever is greater	±15% of detected depth	Horizontal and vertical location of the utility detected by multiple geophysical techniques used. ⁵⁾
		QL-B2P	Yes			
		QL-B1	No	±150 mm or ±15% of detected depth whichever is greater	±25 mm	Horizontal and vertical location of the top and/or bottom of the utility.
		QL-B1P	Yes			
A	Verification	QL-A	-	±50 mm	±25 mm	

¹⁾ Horizontal location is to the centreline of the utility.
²⁾ Vertical location is to the top of the utility.
³⁾ For detection, it is a requirement that a minimum of GPR and EML techniques are used (see 8.2.1.1.2).
⁴⁾ Electronic depth readings using EML equipment are not normally sufficient to achieve a QL-B2 or higher.
⁵⁾ Some utilities can only be detected by one of the existing detection techniques. As a consequence, such utilities cannot be classified as a QL-B1.

Method ¹⁾ (to be determined in consultation with the client)	Survey Grid/Search Resolution 2)				Quality Levels achievable	Typical Application (informative)
	EML ³⁾	GPR		Other Techniques ⁴⁾		
		General	Post- Processing			
M1	Orthogonal search transect at ≤10 m intervals and when following a utility trace, search transects at ≤5 m intervals	Use as applicable	No	≤5 m survey grid	B1, B2, B3, B4	Used where the density of services is typical of an undeveloped area
M1P			Yes		B1P, B2P, B3P	
M2	Orthogonal search transect at ≤5 m intervals and when following a utility trace, search transects at ≤2 m intervals	Either: a) ≤2 m orthogonal; or b) high density array ⁵⁾	No	≤2 m survey grid	B1, B2, B3, B4	Used where the density of services is typical of a suburban area or where the utility services cross a boundary of a survey area
M2P			Yes		B1P, B2P, B3P	
M3	Orthogonal search transect at ≤2 m intervals and when following a utility trace, search transects at ≤1 m intervals	Either: a) ≤1 m orthogonal; or b) high density array ⁵⁾	No	≤1 m survey grid	B1, B2, B3, B4	Used where the density of services is typical of a busy urban area or for clearance surveys prior to operations such as borehole/drilling/fencing/tree planting
M3P			Yes		B1P, B2P, B3P	
M4	Orthogonal search transect at ≤2 m intervals and when following a utility trace, search transects at ≤0.5 m intervals	Either: a) ≤0.5 m orthogonal; or b) high density array ⁵⁾	No	≤0.5 m survey grid	B1, B2, B3, B4	Used where the density of services is typical of a congested city area
M4P			Yes		B1P, B2P, B3P	
NOTE 1 In general the effort increases from M1 to M4 and the addition of post-processing. For areas with a greater density of utilities or areas considered high risk by the client, a detection method that has a higher level of effort should be selected.						
NOTE 2 “p” indicates off-site post-processing has been included.						
1) It is a requirement that a minimum of GPR and EML techniques are used.						
2) The tolerance for orthogonal transect centres and survey grids shall be ±0.1 m.						
3) It is a requirement that passive EML is deployed over the whole survey area and that where an active EML method can be used, it is used.						
4) The transect centre depends on technique used.						
5) A high density array comprises 100 mm or closer antenna separation.						

4 Disclaimers

The survey aims to map all existing utilities and sub-surface structures and provide information with respect to pipe size, material type and drainage connectivity. However, GPR surveying is limited by the following guidelines and it may not be possible to accurately survey, define and locate all services and sub-surface features. Survey Results are representative of the date and time of survey only.

- Locational accuracy is determined by referring to the manufacturers guidelines for the detectors used.
- Existing record information showing underground services is often incomplete and unknown accuracy; therefore, it should be regarded only as an indication.
- In ideal conditions these spatial accuracies for the underground utilities are +/- 5% for the RD4000 and +/- 10% of depth for the GPR to 2.5m deep. However, variations within the subsurface may alter this estimated accuracy.
- Although all reasonable steps have been taken to locate all features, there is no guarantee that all will be shown on the drawing as some above ground features may have obstructed the survey.
- GPR surveying operates best within high resistivity material. Clay overburden can impair GPR surveying.
- Due to the attenuation of the radar signal with depth, resolution is restricted, hence making identification of anomalies difficult with increasing depth.
- The depth penetration and quality of the data depends on the ground conditions on the site. Poor data may be a result of areas with high conductivity. Also, high reflective materials close to the surface i.e., rebar may hide deeper anomalies.
- It is not always possible to trace the entire length of each underground service.
- It is always our intention to use the Utility providers' details, if supplied prior to survey commencement as a guide for location purposes. However, should we not be able to locate those guided services we shall not be held responsible for the accuracy, or otherwise, of the location of that service, as issued by the utility provider and therefore shown "Taken from Records" on the drawing and we are not liable for any loss that may arise due to the lack of accuracy in the guided information.
- Unless otherwise stated, all services and sub-surface structures shown on Murphy Geospatial plan drawings have been surveyed using approved detectors and the connections between manholes, if not traced, are assumed to run straight.
- Plan accuracies of the order of + or - 150mm may be achieved but this figure will depend on the depth of the service below ground level. Where similar services run on close proximity, separation may be impossible. Successful tracing of non-metallic pipes may be limited.
- Please note that not all buried pipes, cables and ducts can be detected and mapped in consideration of their depth, location, material type, geology and proximity to other

utilities. Even an appropriate and professionally executed survey may not be able to achieve a 100% detection rate.

- Services which have been untraceable are shown from Records where possible.
- DP represents distance from the surface level to the top of the service/ radar.

No allowance has been made within our quotation, unless otherwise stated, for the location and mapping of undeclared services. Failure to detect or fully map any declared service will be recorded within the notes accompanying our final drawings.

Where technically possible, depth indications will be given. These should be used for guidance only and wherever critical accuracy is required these should be confirmed by the Client by undertaking trial excavations or similar. Bends, lateral service connections, or the close proximity of other services and local magnetic, atmospheric or ground conditions, could in certain situations influence the accuracy of the plan and depth indication facility. Depths will not be provided unless we are reasonably confident of their validity.

Where Murphy Geospatial issues a CAD drawn utility service plan, this should be read in conjunction with all available public utility records etc. As part of our exhaustive Quality Control procedures, Murphy Geospatial endeavour to add relevant Public Utility record information onto the final issue drawing. An allowance should be made for the width of services, particularly where these are laid in bands or are of significant size etc. For clarification or appropriate easement bands, we would recommend that direct contact is made with the Asset Owner or Statutory Undertaker.

We exclude the following, except where otherwise specified and possible to do so:

- All private service connections, (including water or gas fittings where no through flow of applied signal is possible).
- Pot ended or disconnected cables or terminated short lengths of pipe.
- Internal building services
- Fibre optic cables (except where laid with a standard communications cable or built-in tracer wire or similar conductor system) or can be clearly located using ground penetrating radar.
- Small diameter cables less than 17mm diameter, or pipes less than 38mm diameter.
- Above ground services unless specifically requested.
- Lifting manhole covers which require longer than 10-minute effort using standard heavy duty lifting apparatus.
- Services positioned directly below other pipes or cables etc (i.e., masking signal) - intrusive verification options available on request.
- Deep non-metallic pipes, ducts or culverts (unless probing or Pipe Track 3d is specified as part of the fully invasive survey option).

- Passing through defective pipework (displaced joints etc) or acute bends between access points.

Please note that our Quotation does not allow for location of individual service feeds to properties unless reasonable to do so, as access would be required into each property to apply direct connections to inlet points and this would significantly increase the scope of work, survey cost and also cause possible disruption to occupants.

Service provider utility drawings may not be up to date or give sufficient coverage of all areas surveyed, as such extra precaution should be taken when excavation works are carried out on site and it is recommended to contact service providers before commencing any excavation works within surveyed areas.

All work carried out by Murphy Geospatial conforms to the guidelines set out by The Survey Association (TSA).

5 GNI Gas Pipeline Disclaimer

Gas Networks Ireland (GNI), their affiliates and assigns, accept no responsibility for any information contained in this document concerning location and technical designation of the gas distribution and transmission network ("the Information"). Any representations and warranties express or implied, are excluded to the fullest extent permitted by law. No liability shall be accepted for any loss or damage including, without limitation, direct, indirect, special, incidental, punitive or consequential loss including loss of profits, arising out of or in connection with the use of the Information (including maps or mapping data). NOTE: DIAL BEFORE YOU DIG Phone 1850 427 747 or e-mail dig@gasnetworks.ie – The actual position of the gas/electricity distribution and transmission network must be verified on site before any mechanical excavating takes place. If any mechanical excavation is proposed, hard copy maps must be requested from GNI re gas. All work in the vicinity of the gas distribution and transmission network must be completed in accordance with the current edition of the Health & Safety Authority publication, 'Code of Practice for Avoiding Danger from Underground Services' which is available from the Health and Safety Authority (1890 28 93 89) or can be downloaded free of charge at www.hsa.ie.

6 General GPR Limitations

GPR surveying is limited by the following guidelines		Minimizing GPR Limitations
Depth and size of Utility	In good ground conditions and within the depth range of two metres the ability to detect a utility will reduce in diameter by 1mm for each 10mm of depth. i.e., a 200mm pipe can be detected at 2m and a 50mm pipe at 0.5m but a 25mm plastic water service pipe to a house cannot be detected at 1.2m with radar	Murphy Geospatial incorporated Radio Detection surveys in areas where GPR was found to be ineffective.
Shadowing	This can happen where shallow buried utilities hide or mask deeper buried utilities below.	Murphy Geospatial use mutli frequency radar systems to reduce the effect of shadowing.
Soil Condition	GPR surveying operates best within high resistivity material. Clay overburden can impair GPR surveying. The depth penetration and quality of the data depends on the ground conditions on site. Poor data maybe a result of areas with high conductivity	Murphy Geospatial calibrate our GPR Systems for varying soil types on each project.
Plan Accuracies	Plan accuracies of the order of + or – 150mm maybe achieved but this figure will depend on the depth of the service below ground level.	Murphy Geospatial incorporated Radio Detection surveys in areas where GPR was found to be ineffective.
Utility location	Although all reasonable steps have been taken to locate all features, there is no guarantee that all will be shown on the drawing as some above ground features may have obstructed the survey.	Murphy Geospatial utility surveyors are all qualified and certified to locate underground services.
Existing Utility Records	Existing record information showing underground services is often incomplete and unknown accuracy; therefore, it should be regarded only as an indication.	It is always our intention to use the Utility provider's details, if supplied prior to survey commencement, as a guide for location purposes. However, should we not be able to locate those guided services we shall not be held responsible for the accuracy, or otherwise, of the location of that service, as issued by the utility provider and therefore

		shown "Taken from Records" on the drawing and we are not liable for any loss that may arise due to the lack of accuracy in the guided information.
Loss of Signal	It is not always possible to trace the entire length of each underground service.	Murphy Geospatial will indicate on the drawing if a service trace is lost.
Utility Congestion	Where similar services run on close proximity, separation maybe impossible.	Murphy Geospatial incorporated Radio Detection surveys in areas where GPR was found to be ineffective.
Pipe Material	Successful tracing of non-metallic pipes maybe limited due to material construction of the pipe.	Murphy Geospatial incorporate Radio Detection/ Manhole & PWG surveys in areas where GPR was found to be ineffective.

The American Society of Civil Engineers in their 'Standard Guidance for the collection and depiction of existing subsurface utility data' has a useful rule of thumb for GPR which in, metric values, can be summarised as: 'In good ground conditions and within the depth range of two metres the ability to detect a utility will reduce in diameter by 1mm for each 10mm of depth. i.e., a 200mm pipe can be detected at 2m and a 50mm pipe at 0.5m but a 25mm plastic water service pipe to a house cannot be detected at 1.2m with radar'.

Appendix J : Site Investigation



CAUSEWAY
— GEOTECH

Richmond Road Phase 2 – Ground Investigation

Client: Malkey Limited

Client's Representative: DBFL Consulting Engineers

Report No.: 22-1762

Date: January 2023

Status: Interim Report

This report is an interim report to be used for indicative purposes only and should not be used for detailed design or tendering purposes.

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Document Control Sheet

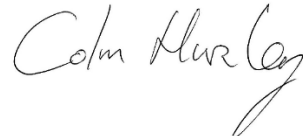
Note on: Methods of describing soils and rocks & abbreviations used on exploratory hole logs

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APPENDICES

Appendix A	Site and exploratory hole location plans
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Document Control Sheet

Report No.:		22-1762			
Project Title:		Richmond Road Phase 2			
Client:		Malkey Limited			
Client's Representative:		DBFL Consulting Engineers			
Revision:	A00	Status:	Interim Report	Issue Date:	20 th January 2023
Prepared by:		Reviewed by:		Approved by:	
 Sean Ross BSc MSc MIEI PGeo		 Colm Hurley BSc PGeo		 Darren O'Mahony BSc MSc MIEI EurGeol PGeo	

The works were conducted in accordance with:

British Standards Institute (2015) BS 5930:2015+A1:2020, Code of practice for ground investigations.

BS EN 1997-2: 2007: Eurocode 7 - Geotechnical design - Part 2 Ground investigation and testing.

Geotechnical Society of Ireland (2016), Specification & Related Documents for Ground Investigation in Ireland

Laboratory testing was conducted in accordance with:

British Standards Institute BS 1377:1990 parts 2, 4, 5, 7 and 9

METHODS OF DESCRIBING SOILS AND ROCKS

Soil and rock descriptions are based on the guidance in BS5930:2015+A1:2020, The Code of Practice for Ground Investigation.

Abbreviations used on exploratory hole logs	
U	Nominal 100mm diameter undisturbed open tube sample (thick walled sampler).
UT	Nominal 100mm diameter undisturbed open tube sample (thin walled sampler).
P	Nominal 100mm diameter undisturbed piston sample.
B	Bulk disturbed sample.
LB	Large bulk disturbed sample.
D	Small disturbed sample.
C	Core sub-sample (displayed in the Field Records column on the logs).
L	Liner sample from dynamic sampled borehole.
W	Water sample.
ES / EW	Soil sample for environmental testing / Water sample for environmental testing.
SPT (s)	Standard penetration test using a split spoon sampler (small disturbed sample obtained).
SPT (c)	Standard penetration test using 60 degree solid cone.
(x,x/x,x,x,x)	Blows per increment during the standard penetration test. The initial two values relate to the seating drive (150mm) and the remaining four to the 75mm increments of the test length.
(Y for Z/ Y for Z)	Incomplete standard penetration test where the full test length was not achieved. The blows 'X' represent the total blows for the given seating or test length 'Z' (mm).
N=X	SPT blow count 'N' given by the summation of the blows 'X' required to drive the full test length (300mm).
HVP / HVR	In situ hand vane test result (HVP) and vane test residual result (HVR). Results presented in kPa.
V VR	Shear vane test (borehole). Shear strength stated in kPa. V: undisturbed vane shear strength VR: remoulded vane shear strength
Soil consistency description	In cohesive soils, where samples are disturbed and there are no suitable laboratory tests, N values may be used to indicate consistency on borehole logs – a median relationship of $N \times 5 = C_u$ is used (as set out in Stroud & Butler 1975).
dd-mm-yyyy	Date at the end and start of shifts, shown at the relevant borehole depth. Corresponding casing and water depths shown in the adjacent columns.
▽	Water strike: initial depth of strike.
▼	Water strike: depth water rose to.
Abbreviations relating to rock core – reference Clause 36.4.4 of BS 5930: 2015+A1:2020	
TCR (%)	Total Core Recovery: Ratio of rock/soil core recovered (both solid and non-intact) to the total length of core run.
SCR (%)	Solid Core Recovery: Ratio of solid core to the total length of core run. Solid core has a full diameter, uninterrupted by natural discontinuities, but not necessarily a full circumference and is measured along the core axis between natural fractures.
RQD (%)	Rock Quality Designation: Ratio of total length of solid core pieces greater than 100mm to the total length of core run.
FI	Fracture Index: Number of natural discontinuities per metre over an indicated length of core of similar intensity of fracturing.
NI	Non Intact: Used where the rock material was recovered fragmented, for example as fine to coarse gravel size particles.
AZCL	Assessed zone of core loss: The estimated depth range where core was not recovered.
DIF	Drilling induced fracture: A fracture of non-geological origin brought about by the rock coring.
(xxx/xxx/xxx)	Spacing between discontinuities (minimum/average/maximum) measured in millimetres.

Richmond Road Phase 2

1 AUTHORITY

On the instructions of DBFL Consulting Engineers, (“the Client’s Representative”), acting on the behalf of Malkey Limited (“the Client”), a ground investigation was undertaken at the above location to provide geotechnical and environmental information for input to the design and construction of a proposed residential development.

This report details the work carried out both on site and in the geotechnical and chemical testing laboratories; it contains a description of the site and the works undertaken, the exploratory hole logs and the laboratory test results. A discussion on the recommendations for construction is also provided.

All information given in this report is based upon the ground conditions encountered during the ground investigation works, and on the results of the laboratory and field tests performed. However, there may be conditions at the site that have not been taken into account, such as unpredictable soil strata, contaminant concentrations, and water conditions between or below exploratory holes. It should be noted that groundwater levels usually vary due to seasonal and/or other effects and may at times differ to those recorded during the investigation. No responsibility can be taken for conditions not encountered through the scope of work commissioned, for example between exploratory hole points, or beneath the termination depths achieved.

This report was prepared by Causeway Geotech Ltd for the use of the Client and the Client’s Representative in response to a particular set of instructions. Any other parties using the information contained in this report do so at their own risk and any duty of care to those parties is excluded.

2 SCOPE

The extent of the investigation, as instructed by the Client’s Representative, included trial pits, soil sampling, environmental sampling, in-situ and laboratory testing, and the preparation of a report on the findings including recommendations for construction.

NOTE: The full scope of works has not been completed for the site.

3 DESCRIPTION OF SITE

As shown on the site location plan in Appendix A, the works were conducted in the car park of Leydens Wholesalers & Distributors on Richmond Road in Drumcondra in Dublin. The site is bordered by Richmond Road to the north, and industrial units on all other sides. The Tolka River is located 40m to the south of the site.

4 SITE OPERATIONS

4.1 Summary of site works

Site operations, which were conducted on the 20th December 2022, comprised:

- four machine dug trial pits
- an infiltration test performed in two trial pits
- plate load testing at two locations.

The exploratory holes and in-situ tests were located as instructed by the Client's Representative, and as shown on the exploratory hole location plan in Appendix A.

4.2 Trial Pits

Four trial pits (TP01–TP02 and IT01–IT02) were excavated using a JCB 3CX fitted with a 600mm wide bucket, to a maximum depth of 1.70m. IT01 and IT02 were excavated to allow completion of infiltration test.

Environmental samples were taken at standard depths in each trial pit.

Disturbed (bulk bag) samples were taken at standard depth intervals and at change of strata.

Any water strikes encountered during excavation were recorded along with any changes in their levels as the excavation proceeded. The stability of the trial pit walls was noted on completion.

Appendix B presents the trial pit logs with photographs of the pits and arising provided in Appendix C.

4.3 Infiltration tests

An infiltration/soakaway test was carried out at two locations (IT01- IT02) in accordance with BRE Digest 365 - Soakaways (BRE, 2016). The tests were conducted in similarly numbered trial pits.

Appendix D presents the results and analysis of the infiltration test.

4.4 Plate load tests

Plate load tests were carried out at two locations (CBR01 and CBR02). Hardstanding at both locations was completed using a JCB3X excavator and the tests undertaken on the sub-base/made ground beneath the hardstanding.

The plate load tests were conducted as incremental loading tests in accordance with Clause 4.1 of BS1377:

Part 9: 1990 (British Standards Institute, 1990). A 450mm diameter bearing plate was used with five equal loadings to a maximum pressure of approximately 300kPa, followed by unloading. The testing was conducted using a wireless plate load testing system, PLATEMAN, which utilises Bluetooth technology with a remotely-operated rugged PDA system.

Plate movements were measured using three strain gauges fitted to a remotely fixed tripod frame. Each loading increment was maintained until the plate movement had essentially stopped.

The test results provided in Appendix E are as follows:

- plots of the plate settlements, average of the three gauges, against pressure.
- plots of average settlement against time during the loading increments/decrement.

The Modulus of Subgrade Reaction, k , is estimated by applying a “best fit” to the settlement-pressure plots, and is reported in MPa/m. The numerical value represents the pressure, in kPa, on the bearing plate that induces 1.25mm of settlement.

An approximate CBR value was estimated using the guidance provided in the Interim Advice Note 73/06 (Revision 1, 2009) of the Design Guidance for Road Pavement Foundations (Draft HD25). The document provides methods to convert the measured k value to the equivalent for a 762mm diameter plate and the consequent relationship with CBR. This method of estimating an equivalent CBR value is relatively conservative.

5 LABORATORY WORK

5.1 Environmental laboratory testing of soils

Environmental testing, was conducted on selected environmental soil samples by Chemtest at its laboratory in Newmarket, Suffolk.

Rilta suite of analysis was carried out on seven samples for landfill disposal criteria. This included testing for a range of determinants, including:

Testing was carried out for a range of determinants, including:

- Metals
- Speciated total petroleum hydrocarbons (TPH)
- Speciated polycyclic aromatic hydrocarbons (PAH)
- BTEX compounds
- Volatile Organic Compounds (VOCs)
- Semi-Volatile Organic Compounds (SVOCs)

- Polychlorinated biphenyls (PCBs)
- Phenols
- Organic matter
- Total Organic Carbon (TOC)
- Cyanides
- Asbestos screen
- Sulphate and sulphide
- Sulphur
- Phosphate
- Calcium
- pH
- Waste acceptance criteria (WAC)

Results of environmental laboratory testing are presented in Appendix F.

A waste classification report was compiled analysing the results of the above testing. The report is presented in Appendix G.

6 GROUND CONDITIONS

6.1 General geology of the area

Published geological mapping indicate the superficial deposits underlying the site comprise made ground and alluvium. These deposits are underlain by limestones and shales of the Lucan Formation.

6.2 Ground types encountered during investigation of the site

A summary of the ground types encountered in the exploratory holes is listed below, in approximate stratigraphic order:

- **Paved surface:** All location encountered 100mm of bitmac surfacing.
- **Made Ground (fill):** greyish brown sandy clayey gravel encountered immediately below bitmac containing anthropogenic material such as concrete and brick fragments, underlain by reworked sandy gravelly clay which also contained fragments of brick, concrete, scrap metal and Styrofoam.

6.3 Groundwater

Details of the individual groundwater strikes, along with any relative changes in levels as works proceeded, are presented on the exploratory hole logs for each location.

Groundwater was encountered during excavation of TP01 and IT01 as groundwater strikes at depths of 1.50m and 1.30m respectively. Groundwater was not noted during excavation of either of the other pits.

Seasonal variation in groundwater levels should also be factored into design considerations.

7 DISCUSSION

7.1 Proposed construction

It is proposed to construct a new residential development on the site with associated infrastructure.

No further details were available to Causeway Geotech at the time of preparing this report and any designs based on the recommendations or conclusions within this report should be completed in accordance with the current design codes, taking into account the variation and the specific details contained within the exploratory holes. Causeway Geotech were commissioned to provide a geotechnical report, and it is outwith our remit to advise on structure design.

7.2 Recommendations for construction

7.2.1 Access roads, car parks and hard standing

Based on a summary of the CBR tests undertaken at the site, it is envisaged that the upper strata on site may be suitable for the placement of road make up layers however it should be noted that the two results (0.7% and 30%) are likely representative of the variation in made ground encountered across the site within which the two tests were conducted. Ideally this made ground should be excavated and replaced with engineered backfill for pavement build up.

Table 2.1 of volume 7 section 2 of the Design Manual for Roads and Bridges (below), gives guidance on the average thickness of the pavement layers in relation to the CBR results. As can be seen, a CBR in excess of 15% does not require any capping layers, however a sub-base thickness of 200mm is suggested.

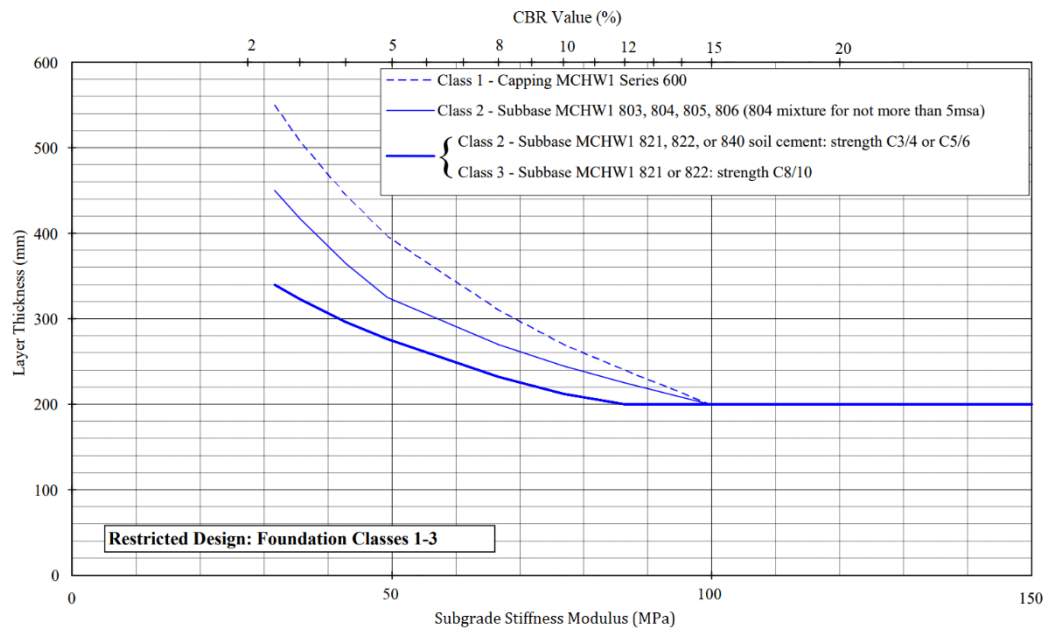


Table 2.1 (DMRB Vol.7 Sec2) 2009

It is recommended that further testing be undertaken during the course of construction works at intervals as set out in the Earthworks Specification, and should any areas indicate lower than expected value, the above plot should be used to determine the thicknesses of any capping or sub-base layers that may need to be placed in these areas.

The use of geosynthetics in the construction of paved areas, will be beneficial, particularly in areas of Made Ground. These could include a geosynthetic (e.g., a geogrid) at subgrade level with further benefit gained by incorporating further layer(s) within the capping/sub-base layer. Road design should be undertaken by a specialist earthworks contractor/designer.

7.3 Infiltration drainage

In infiltration test carried out in trial pit IT01 and IT02, the rates of infiltration were calculated as shown in Table 1 below.

Table 1 Summary of BRE soakaway test results

GI Ref	Infiltration rate (q)	Strata
IT01	n/a	Made Ground: Reworked sandy gravelly CLAY.
IT02	0.169	Made Ground: Reworked sandy gravelly CLAY.

The low-permeability fine-grained soils encountered in IT01 along with the soil descriptions, are therefore considered to be poor infiltration media, and would be deemed unsuitable for the implementation of infiltration drainage systems.

The rate of infiltration calculated in IT02 along with the material descriptions imply that the subsoil may be considered suitable media for an infiltration drainage system.

Reference should be made the Sustainable Drainage Systems (SuDS) design guidance, taking into account meteorological conditions and a hydrogeological assessment.

7.4 Waste classification

For consideration of material to be removed from site, a waste classification of the solid soil laboratory results was completed using HazWasteOnline™ software. A copy of the Waste Classification report is included at Appendix G. The Waste Classification report shows that the material tested can be classified as non-hazardous material considering the List of Wastes (LoW) code 17 for Construction and Demolition Wastes (including soils excavated from contaminated sites), specifically 17 05 03* and 17 05 04.

Following completion of the waste classification, and to determine a suitable disposal route for the soil, assessment of the WAC analysis of the samples was completed. The laboratory results of the WAC testing indicate that the soils from the site are suitable for disposal as Inert waste to an appropriate licenced facility with the exception of those samples listed in Table 2 which exceeded Inert WAC limits.

Table 2 Samples where Non-Haz WAC are exceeded

GI Ref	Determinant Failure	Comment
IT01 0.5m	-TOC (3.5%) exceeds Inert WAC (3.5%) -Antimony (0.074mg/kg) exceed Inert WAC (0.06%)	Exceeds Inert WAC, suitable for disposal as non-hazardous waste.
IT01 1.5m	-TOC (3.4%) exceeds Inert WAC (3.5%) -Antimony (0.064mg/kg) exceed Inert WAC (0.06%)	Exceeds Non-Haz WAC, suitable for disposal as hazardous waste.
TP01 1.5m	-TOC (3.3%) exceeds Inert WAC (3%)	Exceeds Non-Haz WAC, suitable for disposal as hazardous waste. (see Note 1)

Note 1: Per Section 2 paragraph 2.1.2.2 of EU Council Decision 2003-33-EC higher value TOC may be admitted by the competent authority for Inert WAC provided the DOC value of <500 mg/kg is achieved. It is recommended that the receiving destination review WAC test data to ensure suitability for disposal in line with licensing arrangements.

It is noted that this waste classification assessment has been based solely on the available samples results and corresponding investigation findings. In making this assessment all due care and attention to available and relevant legislative and guidance frameworks has been taken in arriving at the conclusions.

Also, potential areas of localised contamination outside the areas of the investigation cannot be discounted. Any potential contamination identified during site development work by visual or olfactory means should be investigated, including further laboratory testing, and appropriate health & safety, waste disposal and remediation measures adopted. Additional testing of the soils to be disposed from site may also be requested by the individual landfill before acceptance at their facility.

8 REFERENCES

Geotechnical Society of Ireland (2016), Specification & Related Documents for Ground Investigation in Ireland.

IS EN 1997-2: 2007: Eurocode 7 - Geotechnical design - Part 2 Ground investigation and testing. National Standards Authority of Ireland.

BS 5930: 2015+A1:2020: Code of practice for ground investigations. British Standards Institution.

BS EN ISO 14688-1:2018: Geotechnical investigation and testing. Identification and classification of soil. Part 1 Identification and description.

BS EN ISO 14688-2:2018: Geotechnical investigation and testing. Identification and classification of soil. Part 2 Principles for a classification.

BS 1377: 1990: Methods of test for soils for civil engineering purposes. British Standards Institution.

Building Research Establishment (2007), BRE Digest 365: Soakaways.

Land contamination risk management (LCRM), (2020) Environment Agency.



CAUSEWAY
— GEOTECH

APPENDIX A
SITE AND EXPLORATORY HOLE LOCATION PLANS





Project No.: 22-1762
Project Name: Richmond Road Phase 2

Client: Malkey Limited
Client's Representative: DBFL Consulting Engineers

Legend Key



Title:
Site Location Plan

Last Revised:
20/01/2023

Scale:
1:10000



Project No.: 22-1762
Project Name: Richmond Road Phase 2

Client: Malkey Limited
Client's Representative: DBFL Consulting Engineers

Legend Key

-  Locations By Type - PLT
-  Locations By Type - TP



Title:
Site Location Plan

Last Revised:
20/01/2023

Scale:
1:750

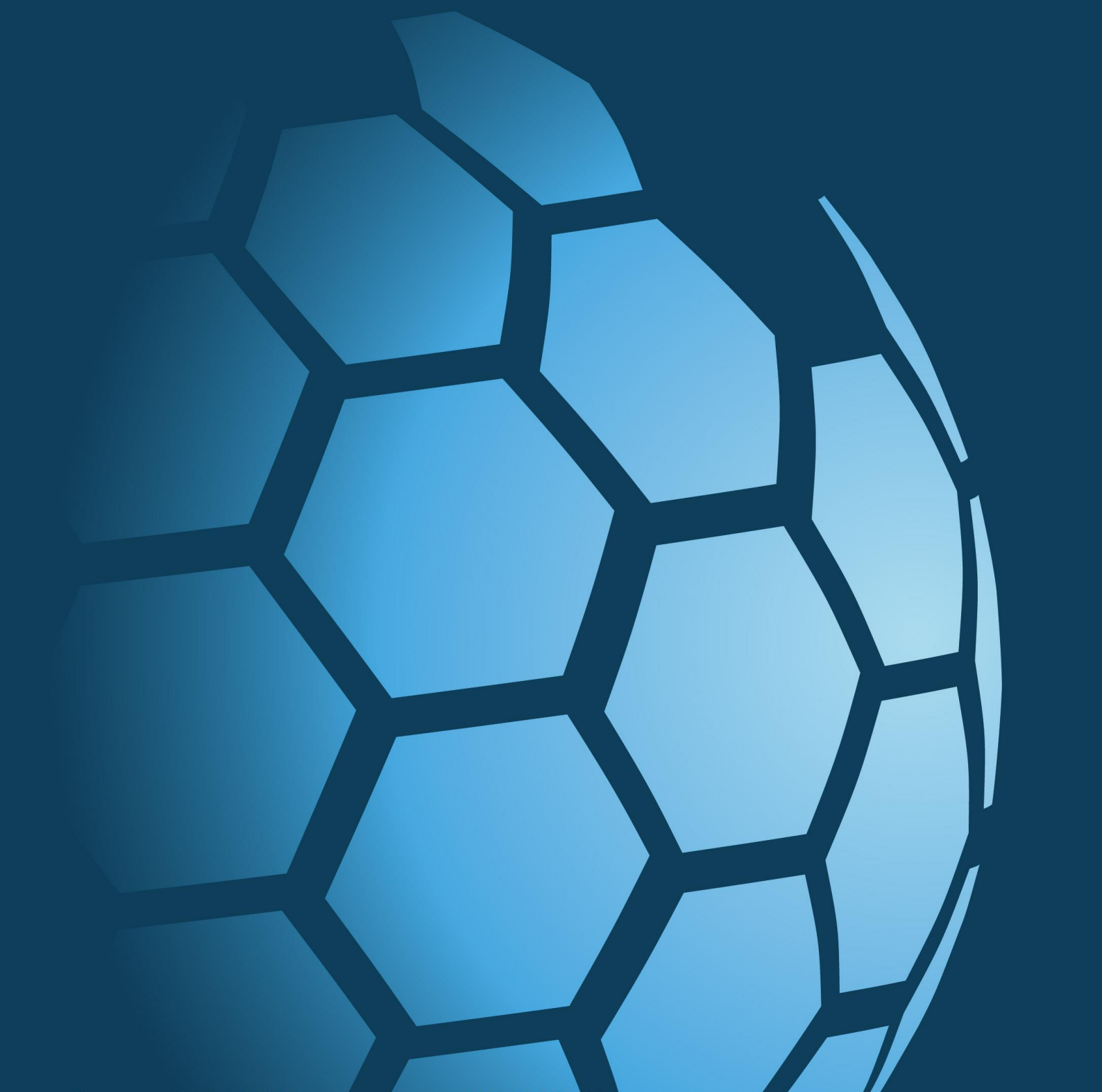


Microsoft product screen shot(s) reprinted with permission from Microsoft Corporation



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APPENDIX B
TRIAL PIT LOGS





22-1762

Richmond Road Phase 2

TP01

Trial Pitting

716607.86 E

736468.04 N

Malkey Limited

DBFL Consulting Engineers

Scale: 1:25

JCB 3CX

mOD

20/12/2022

DM

FINAL

Depth (m)	Sample / Tests	Field Records	Level (mOD)	Depth (m)	Legend	Description	Water
				0.10		BITMAC	
0.50 0.50	B1 ES2					MADE GROUND: Greyish brown sandy clayey angular fine to coarse GRAVEL with fragments of brick and concrete. Sand is fine to coarse.	
				0.75		MADE GROUND: Soft dark brown sandy gravelly CLAY with fragments of concrete, cinder blocks, scrap metal and wire. Sand is fine to coarse. Gravel is angular to subangular fine to coarse.	
1.50 1.50	B3 ES4	Heavy strike at 1.50m.		1.70		End of trial pit at 1.70m	

Water Strikes		Depth: 1.70 Width: 0.70 Length: 2.10	Remarks:		
Struck at (m)	Remarks				
1.50	Heavy strike at 1.50m.				
		Stability:	Termination Reason		Last Updated
		Unstable	Terminated due to water ingress causing the pit to collapse.		20/01/2023
					

 CAUSEWAY GEOTECH			Project No. 22-1762		Project Name: Richmond Road Phase 2			Trial Pit ID TP02			
			Coordinates 716638.20 E 736460.23 N		Client: Malkey Limited Client's Representative: DBFL Consulting Engineers						
Method: Trial Pitting			Elevation mOD		Date: 20/12/2022			Logger: DM		Sheet 1 of 1 Scale: 1:25	
Plant: JCB 3CX										FINAL	
Depth (m)	Sample / Tests	Field Records	Level (mOD)	Depth (m)	Legend	Description				Water	
0.50	B1			0.10		BITMAC					
0.50	ES2					MADE GROUND: Greyish brown sandy angular to subangular fine to coarse GRAVEL with fragments of red brick, concrete and pieces of styrafoam. Sand is fine to coarse.					0.5
				0.60		End of trial pit at 0.60m					1.0
											1.5
											2.0
											2.5
											3.0
											3.5
											4.0
											4.5
Water Strikes			Depth: 0.60		Remarks: Steel plate/ chamber encountered at 0.60m. No groundwater encountered.						
Struck at (m)		Remarks	Width: 0.65								
			Length: 1.70								
			Stability:		Termination Reason				Last Updated		
			Stable		Terminated on Engineer's instruction.				20/01/2023		



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APPENDIX C
TRIAL PIT PHOTOGRAPHS





TP01



TP01



TP01



TP01



TP02



TP02



TP02



TP02



TP02



IT01



IT01



IT01



IT02



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— GEOTECH

APPENDIX D
SOAKAWAY TEST RESULTS



 CAUSEWAY GEOTECH			Project No. 22-1762		Project Name: Richmond Road Phase 2			Trial Pit ID IT01																																								
			Coordinates 716612.97 E 736475.15 N		Client: Malkey Limited Client's Representative: DBFL Consulting Engineers																																											
Method: Trial Pitting			Elevation mOD		Date: 20/12/2022			Logger: DM		Sheet 1 of 1 Scale: 1:25																																						
Plant: JCB 3CX										FINAL																																						
<table><tr><td>Depth (m)</td><td>Sample / Tests</td><td>Field Records</td><td>Level (mOD)</td><td>Depth (m)</td><td>Legend</td><td>Description</td><td>Water</td><td></td></tr><tr><td>0.50 0.50</td><td>B1 ES2</td><td rowspan="3">Strike at 1.30m.</td><td></td><td>0.10</td><td></td><td>TARMAC</td><td rowspan="3"></td><td rowspan="3"></td></tr><tr><td></td><td></td><td>0.50</td><td></td><td>MADE GROUND: Greyish brown sandy angular to subangular fine to coarse GRAVEL with fragments of brick, concrete and pieces of cloth.</td></tr><tr><td>1.50 1.50</td><td>B3 ES4</td><td>1.50</td><td></td><td>MADE GROUND: Soft to firm brownish black sandy gravelly CLAY with fragments of brick and concrete. Sand is fine to coarse.</td></tr><tr><td colspan="6"></td><td colspan="2">End of trial pit at 1.50m</td><td></td><td></td></tr></table>											Depth (m)	Sample / Tests	Field Records	Level (mOD)	Depth (m)	Legend	Description	Water		0.50 0.50	B1 ES2	Strike at 1.30m.		0.10		TARMAC					0.50		MADE GROUND: Greyish brown sandy angular to subangular fine to coarse GRAVEL with fragments of brick, concrete and pieces of cloth.	1.50 1.50	B3 ES4	1.50		MADE GROUND: Soft to firm brownish black sandy gravelly CLAY with fragments of brick and concrete. Sand is fine to coarse.							End of trial pit at 1.50m			
Depth (m)	Sample / Tests	Field Records	Level (mOD)	Depth (m)	Legend	Description	Water																																									
0.50 0.50	B1 ES2	Strike at 1.30m.		0.10		TARMAC																																										
			0.50		MADE GROUND: Greyish brown sandy angular to subangular fine to coarse GRAVEL with fragments of brick, concrete and pieces of cloth.																																											
1.50 1.50	B3 ES4		1.50		MADE GROUND: Soft to firm brownish black sandy gravelly CLAY with fragments of brick and concrete. Sand is fine to coarse.																																											
						End of trial pit at 1.50m																																										
<table><tr><td colspan="2">Water Strikes</td><td rowspan="3">Depth: 1.50 Width: 0.65 Length: 1.80 Stability: Stable</td><td colspan="4" rowspan="3">Remarks: Termination Reason Terminated at scheduled depth.</td><td rowspan="3">Last Updated 20/01/2023</td><td rowspan="3"></td></tr><tr><td>Struck at (m)</td><td>Remarks</td></tr><tr><td>1.30</td><td>Strike at 1.30m.</td></tr></table>											Water Strikes		Depth: 1.50 Width: 0.65 Length: 1.80 Stability: Stable	Remarks: Termination Reason Terminated at scheduled depth.				Last Updated 20/01/2023		Struck at (m)	Remarks	1.30	Strike at 1.30m.																									
Water Strikes		Depth: 1.50 Width: 0.65 Length: 1.80 Stability: Stable	Remarks: Termination Reason Terminated at scheduled depth.				Last Updated 20/01/2023																																									
Struck at (m)	Remarks																																															
1.30	Strike at 1.30m.																																															

Soakaway Infiltration Test

Project No.: 22-1762
Site: Richmond Road Phase 2
Test Location: IT01
Test Date: 20-Dec-22



*Analysis using method as described in BRE Digest 365
and CIRIA Report C697-The SUDS Manual*

width (m) length (m)

test pit top dimensions 0.65 2.10

test pit base dimensions 0.50 1.70

test pit depth (m) 1.60

depth to groundwater before adding water (m) = Dry

time (mins)	depth to water surface (m)	depth of water in pit (m)
0	0.75	0.85
1	0.75	0.85
2	0.75	0.85
4	0.76	0.84
6	0.77	0.83
8	0.78	0.82
10	0.78	0.82
15	0.79	0.81
20	0.79	0.81
25	0.80	0.80
30	0.80	0.80
45	0.80	0.80
60	0.82	0.78
75	0.83	0.77
90	0.84	0.76
120	0.85	0.75

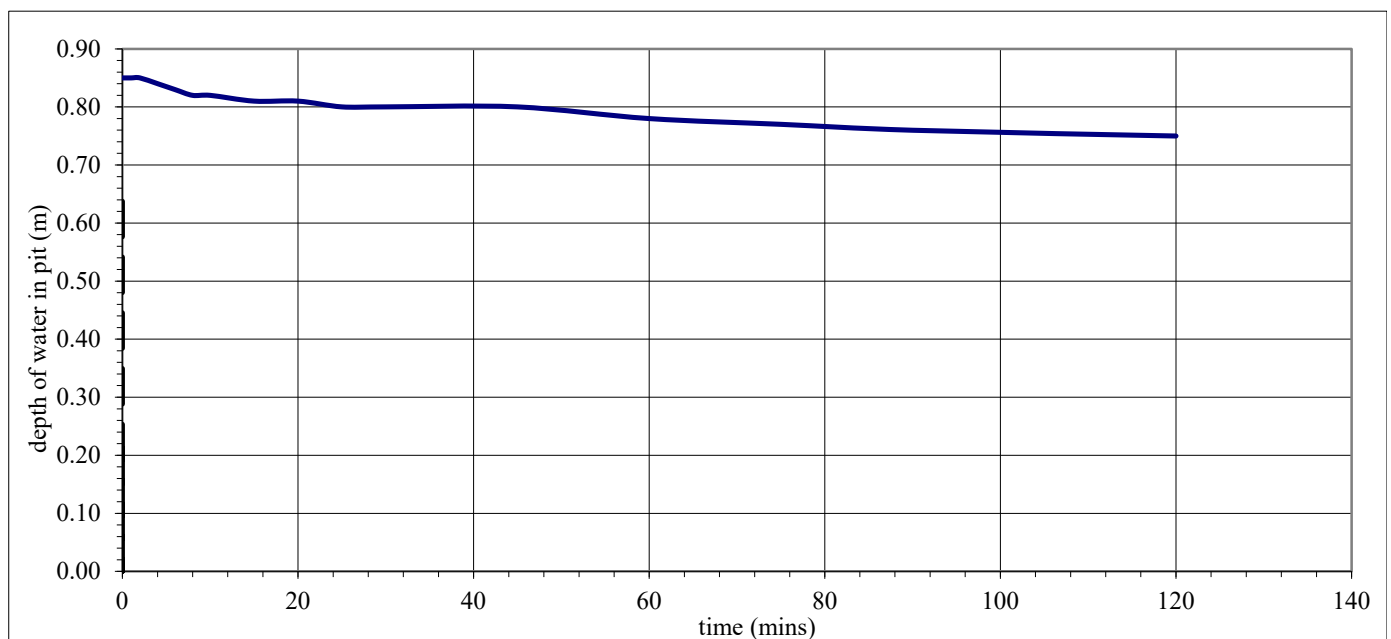
From graph below:

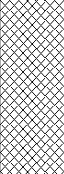
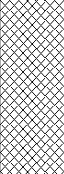
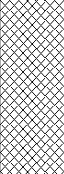
test start - 75% depth at
0.6375 m water depth
time is not determined

test end - 25% depth at
0.2125 m water depth
time is not determined

infiltration rate (q) is very low

time (mins)	depth to water (m)	depth of water in pit (m)	time elapsed (mins)	volume of water lost (m ³)	Area of walls and base at 50% drop (m ²)	q (m/min)	q (m/h)
	0.96	0.6375					
	1.39	0.2125					



 CAUSEWAY GEOTECH			Project No. 22-1762		Project Name: Richmond Road Phase 2			Trial Pit ID IT02																																			
			Coordinates 716643.27 E 736468.91 N		Client: Malkey Limited Client's Representative: DBFL Consulting Engineers																																						
Method: Trial Pitting			Elevation mOD		Date: 20/12/2022			Logger: DM		Sheet 1 of 1 Scale: 1:25																																	
Plant: JCB 3CX								FINAL																																			
<table><tr><td>Depth (m)</td><td>Sample / Tests</td><td>Field Records</td><td>Level (mOD)</td><td>Depth (m)</td><td>Legend</td><td>Description</td><td>Water</td><td></td></tr><tr><td rowspan="2">0.50 0.50</td><td rowspan="2">B1 ES2</td><td rowspan="2"></td><td rowspan="2"></td><td>0.20</td><td></td><td>TARMAC</td><td rowspan="2"></td><td rowspan="2"></td></tr><tr><td>0.80</td><td></td><td>MADE GROUND: Brownish grey sandy angular to subangular fine to coarse GRAVEL. Sand is fine to coarse.</td></tr><tr><td rowspan="2">1.50 1.50</td><td rowspan="2">B3 ES4</td><td rowspan="2"></td><td rowspan="2"></td><td>1.50</td><td></td><td>MADE GROUND: Soft to firm light brown sandy slightly gravelly CLAY with fragments of yellow brick, scrap metal and concrete. Sand is fine to coarse. Gravel is subangular to subrounded fine to medium.</td><td rowspan="2"></td><td rowspan="2"></td></tr><tr><td></td><td></td><td>End of trial pit at 1.50m</td></tr></table>											Depth (m)	Sample / Tests	Field Records	Level (mOD)	Depth (m)	Legend	Description	Water		0.50 0.50	B1 ES2			0.20		TARMAC			0.80		MADE GROUND: Brownish grey sandy angular to subangular fine to coarse GRAVEL. Sand is fine to coarse.	1.50 1.50	B3 ES4			1.50		MADE GROUND: Soft to firm light brown sandy slightly gravelly CLAY with fragments of yellow brick, scrap metal and concrete. Sand is fine to coarse. Gravel is subangular to subrounded fine to medium.					End of trial pit at 1.50m
Depth (m)	Sample / Tests	Field Records	Level (mOD)	Depth (m)	Legend	Description	Water																																				
0.50 0.50	B1 ES2			0.20		TARMAC																																					
				0.80		MADE GROUND: Brownish grey sandy angular to subangular fine to coarse GRAVEL. Sand is fine to coarse.																																					
1.50 1.50	B3 ES4			1.50		MADE GROUND: Soft to firm light brown sandy slightly gravelly CLAY with fragments of yellow brick, scrap metal and concrete. Sand is fine to coarse. Gravel is subangular to subrounded fine to medium.																																					
						End of trial pit at 1.50m																																					
Water Strikes		Depth: 1.50		Remarks: No groundwater encountered.																																							
Struck at (m)	Remarks	Width: 0.70																																									
		Length: 1.80																																									
		Stability: Moderately stable		Termination Reason Terminated at scheduled depth.			Last Updated 20/01/2023																																				

Soakaway Infiltration Test

Project No.: 22-1762
Site: Richmond Road Phase 2
Test Location: IT02
Test Date: 20-Dec-22



*Analysis using method as described in BRE Digest 365
and CIRIA Report C697-The SUDS Manual*

	width (m)	length (m)
test pit top dimensions	0.65	1.90
test pit base dimensions	0.40	1.50
test pit depth (m)	1.60	

depth to groundwater before adding water (m) = Dry

time (mins)	depth to water surface (m)	depth of water in pit (m)
0	1.00	0.60
1	1.07	0.53
2	1.10	0.50
4	1.18	0.42
6	1.22	0.38
8	1.27	0.33
10	1.30	0.30
15	1.33	0.27
20	1.39	0.21
25	1.41	0.19
30	1.43	0.17
45	1.45	0.15
60	1.48	0.12
75	1.50	0.10
90	1.55	0.05
120	1.60	0.00

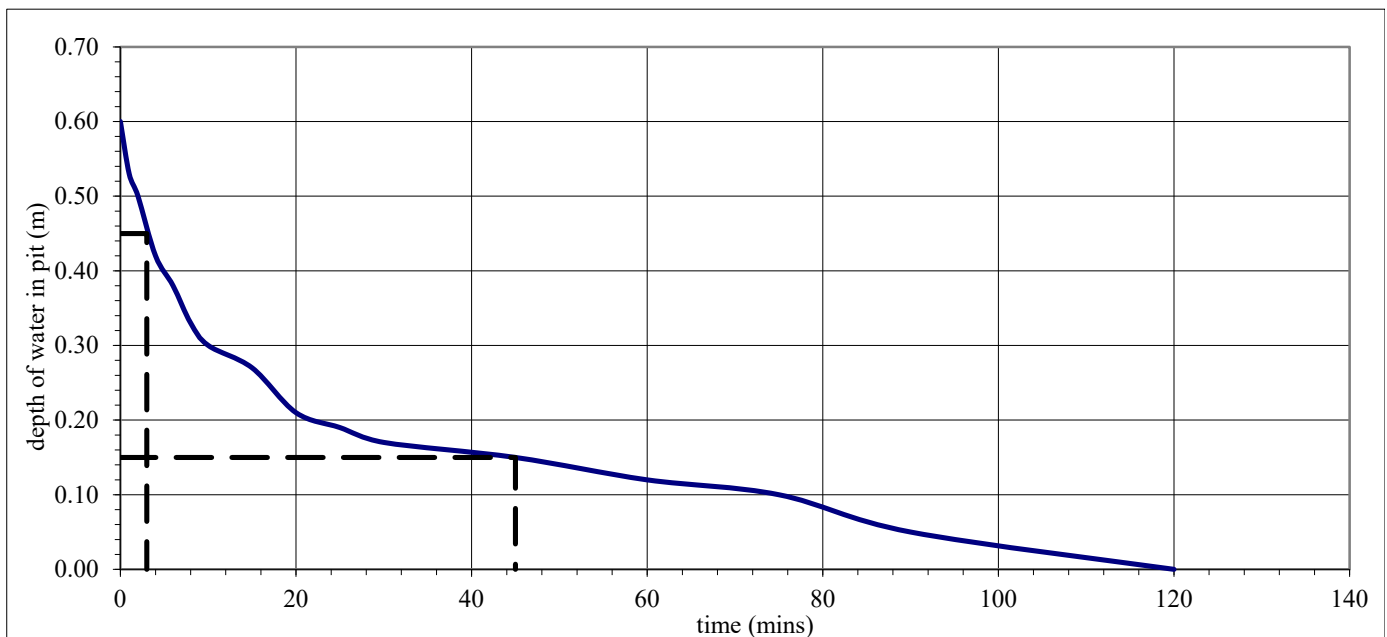
From graph below:

test start - 75% depth at
0.45 m water depth
time is 3.0 minutes

test end - 25% depth at
0.15 m water depth
time is 45.0 minutes

test infiltration rate (q) = 0.169 m/h

time (mins)	depth to water (m)	depth of water in pit (m)	time elapsed (mins)	volume of water lost (m ³)	Area of walls and base at 50% drop (m ²)	q (m/min)	q (m/h)
3	1.15	0.45	42	0.21	1.79	2.8E-03	0.169
45	1.45	0.15					





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APPENDIX E
PLATE LOAD TEST RESULTS

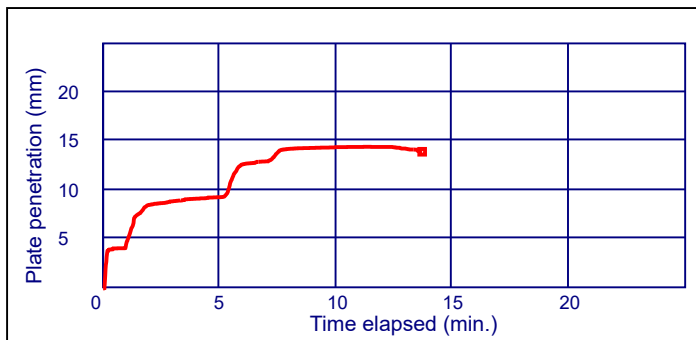


TEST REPORT PLATE LOADING TEST

Project Client	Richmond Rd	Test No:	22-1762
		Lab Ref No:	CBR01
		Date Reported	21.12.22
		Weather Conditions	Dry
Technician	DM	Air Temperature °C	7
Date Tested	20.12.22	Plate Dia (mm)	450
Location		Depth (m)	0.3
GPS Coord's		Reaction Type	15T
Material Type	MG: CLAY	App Weight (kg)	43
No Cycles	1		



Plate Settlement (mm)	Applied Pressure (kN/m2)
0.00	0.0
4.12	77.1
9.36	124.4
13.02	189.6
14.38	213.7
14.51	212.4
14.01	0.0



Maximum Applied Pressure (kPa):	Cycle 1 216
Maximum deformation (mm):	14.51
Modulus of subgrade reaction K (MN/m3):	18.7
K762 (MN/m3):	11.7
Estimated CBR (%):	0.7

Comments: Final loading stage not applied due to high settlement.



Approved Signature
Causeway Geotech
 Darren O'Mahony
 Director

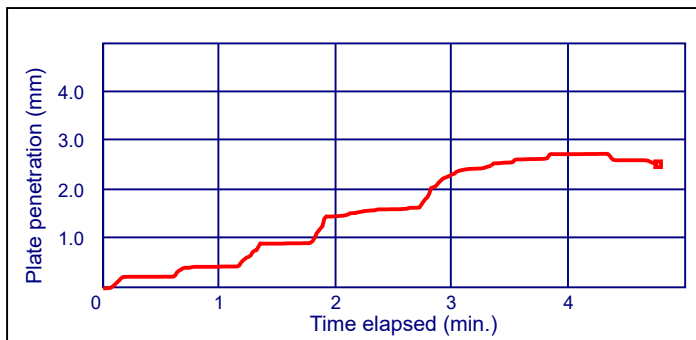
Plate Load - Tested in accordance with
 BS 1377 : Part 9 Cl. 4.1 : 1990

TEST REPORT PLATE LOADING TEST

Project Client	Richmond Rd	Test No:	22-1762
		Lab Ref No:	CBR02
		Date Reported	21.12.22
		Weather Conditions	Dry
Technician	DM	Air Temperature °C	7
Date Tested	20.12.22	Plate Dia (mm)	450
Location		Depth (m)	0.3
GPS Coord's		Reaction Type	15T
Material Type	MG: CLAY	App Weight (kg)	43
No Cycles	1		



Plate Settlement (mm)	Applied Pressure (kN/m²)
0.00	0.0
0.24	78.9
0.45	121.7
0.93	180.6
1.65	239.7
2.75	299.3
2.55	0.0



Maximum Applied Pressure (kPa):
Maximum deformation (mm):
Modulus of subgrade reaction K (MN/m³):
K762 (MN/m³):
Estimated CBR (%):

Cycle 1
302
2.75
165.4
103.1
30

Comments:



Approved Signature
Causeway Geotech
 Darren O'Mahony
 Director

Plate Load - Tested in accordance with
 BS 1377 : Part 9 Cl. 4.1 : 1990



CAUSEWAY
— GEOTECH

APPENDIX F
ENVIRONMENTAL LABORATORY TEST RESULTS



Final Report

Report No.: 22-49100-1

Initial Date of Issue: 18-Jan-2023

Client Causeway Geotech Ltd

Client Address: 8 Drumahiskey Road
Balnamore
Ballymoney
County Antrim
BT53 7QL

Contact(s): Alistair McQuat
Carin Cornwall
Celine Rooney
Colm Hurley
Darren O'Mahony
Dean McCloskey
Gabriella Horan
Joe Gervin
John Cameron
Lucy Newland
Martin Gardiner
Matthew Gilbert
Matthew Graham
Neil Haggan
Paul Dunlop
Sean Ross
Stephen Franey
S

Project 22-1762 Richmond Road Phase 2

Quotation No.: **Date Received:** 23-Dec-2022

Order No.: **Date Instructed:** 23-Dec-2022

No. of Samples: 7

Turnaround (Wkdays): 8 **Results Due:** 10-Jan-2023

Date Approved: 18-Jan-2023

Approved By:



Details: Stuart Henderson, Technical Manager

Results - Soil

Project: 22-1762 Richmond Road Phase 2

Client: Causeway Geotech Ltd	Chemtest Job No.:				22-49100	22-49100	22-49100	22-49100	22-49100	22-49100	22-49100
Quotation No.:	Chemtest Sample ID.:				1569217	1569218	1569219	1569220	1569221	1569222	1569223
Order No.:	Client Sample Ref.:				1	2	1	2	1	2	1
	Sample Location:				IT01	IT01	IT02	IT02	TP01	TP01	TP02
	Sample Type:				SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
	Top Depth (m):				0.5	1.5	0.5	1.5	0.5	1.5	0.5
	Date Sampled:				22-Dec-2022	22-Dec-2022	22-Dec-2022	22-Dec-2022	22-Dec-2022	22-Dec-2022	22-Dec-2022
Determinand	Accred.	SOP	Units	LOD							
Moisture	N	2030	%	0.020	12	29	17	27	9.5	19	17
Arsenic	U	2455	mg/kg	0.5	8.9	7.2	8.1	7.6	12	11	8.7
Barium	U	2455	mg/kg	0	77	55	220	59	48	68	150
Cadmium	U	2455	mg/kg	0.10	0.54	0.78	0.69	0.66	0.79	1.3	0.45
Chromium	U	2455	mg/kg	0.5	5.3	8.9	8.7	8.2	4.1	12	8.5
Molybdenum	U	2455	mg/kg	0.5	2.3	1.9	1.3	1.2	2.1	2.8	0.7
Antimony	N	2455	mg/kg	2.0	< 2.0	< 2.0	< 2.0	< 2.0	2.1	< 2.0	< 2.0
Copper	U	2455	mg/kg	0.50	41	21	19	17	30	28	33
Mercury	U	2455	mg/kg	0.05	0.31	0.22	1.1	0.40	0.11	0.36	1.6
Nickel	U	2455	mg/kg	0.50	11	22	18	17	11	27	11
Lead	U	2455	mg/kg	0.50	150	55	120	60	50	66	260
Selenium	U	2455	mg/kg	0.25	0.66	0.69	0.53	0.49	0.46	1.1	0.25
Zinc	U	2455	mg/kg	0.50	130	68	130	66	140	95	77
Chromium (Trivalent)	N	2490	mg/kg	1.0	5.3	8.9	8.7	8.2	4.1	12	8.3
Chromium (Hexavalent)	N	2490	mg/kg	0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50
Total Organic Carbon	U	2625	%	0.20	3.5	3.4	1.3	1.8	1.2	3.3	0.91
Aliphatic TPH >C5-C6	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C6-C8	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C8-C10	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C10-C12	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C12-C16	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C16-C21	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C21-C35	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C35-C44	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Aliphatic Hydrocarbons	N	2680	mg/kg	5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0
Aromatic TPH >C5-C7	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C7-C8	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C8-C10	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C10-C12	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C12-C16	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C16-C21	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C21-C35	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C35-C44	N	2680	mg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Aromatic Hydrocarbons	N	2680	mg/kg	5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0
Total Petroleum Hydrocarbons	N	2680	mg/kg	10.0	< 10	< 10	< 10	< 10	< 10	< 10	< 10
Benzene	U	2760	µg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Toluene	U	2760	µg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Ethylbenzene	U	2760	µg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
m & p-Xylene	U	2760	µg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0

Results - Soil

Project: 22-1762 Richmond Road Phase 2

Client: Causeway Geotech Ltd	Chemtest Job No.:				22-49100	22-49100	22-49100	22-49100	22-49100	22-49100	22-49100
Quotation No.:	Chemtest Sample ID.:				1569217	1569218	1569219	1569220	1569221	1569222	1569223
Order No.:	Client Sample Ref.:				1	2	1	2	1	2	1
	Sample Location:				IT01	IT01	IT02	IT02	TP01	TP01	TP02
	Sample Type:				SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
	Top Depth (m):				0.5	1.5	0.5	1.5	0.5	1.5	0.5
	Date Sampled:				22-Dec-2022	22-Dec-2022	22-Dec-2022	22-Dec-2022	22-Dec-2022	22-Dec-2022	22-Dec-2022
Determinand	Accred.	SOP	Units	LOD							
o-Xylene	U	2760	µg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Methyl Tert-Butyl Ether	U	2760	µg/kg	1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Naphthalene	U	2800	mg/kg	0.10	0.25	0.11	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Acenaphthylene	N	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Acenaphthene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Fluorene	U	2800	mg/kg	0.10	0.15	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Phenanthrene	U	2800	mg/kg	0.10	1.0	0.42	0.23	0.17	0.19	0.23	0.16
Anthracene	U	2800	mg/kg	0.10	0.22	0.11	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Fluoranthene	U	2800	mg/kg	0.10	1.3	0.66	0.29	< 0.10	0.34	0.24	0.18
Pyrene	U	2800	mg/kg	0.10	1.1	0.64	0.24	< 0.10	0.37	0.26	0.17
Benzo[a]anthracene	U	2800	mg/kg	0.10	0.65	0.39	< 0.10	< 0.10	< 0.10	0.14	< 0.10
Chrysene	U	2800	mg/kg	0.10	0.60	0.32	< 0.10	< 0.10	< 0.10	0.12	< 0.10
Benzo[b]fluoranthene	U	2800	mg/kg	0.10	0.91	0.59	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Benzo[k]fluoranthene	U	2800	mg/kg	0.10	0.25	0.24	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Benzo[a]pyrene	U	2800	mg/kg	0.10	0.65	0.41	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Indeno(1,2,3-c,d)Pyrene	U	2800	mg/kg	0.10	0.46	0.35	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Dibenz(a,h)Anthracene	N	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Benzo[g,h,i]perylene	U	2800	mg/kg	0.10	0.55	0.30	< 0.10	< 0.10	0.18	< 0.10	< 0.10
Coronene	N	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Total Of 17 PAH's	N	2800	mg/kg	2.0	8.1	4.5	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0
PCB 28	U	2815	mg/kg	0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
PCB 52	U	2815	mg/kg	0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
PCB 90+101	U	2815	mg/kg	0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
PCB 118	U	2815	mg/kg	0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
PCB 153	U	2815	mg/kg	0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
PCB 138	U	2815	mg/kg	0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
PCB 180	U	2815	mg/kg	0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
Total PCBs (7 Congeners)	U	2815	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10

Results - Single Stage WAC

Project: 22-1762 Richmond Road Phase 2

Chemtest Job No: 22-49100					Landfill Waste Acceptance Criteria		
Chemtest Sample ID: 1569217					Limits		
Sample Ref: 1					Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Sample ID:							
Sample Location: IT01							
Top Depth(m): 0.5							
Bottom Depth(m):							
Sampling Date: 22-Dec-2022							
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	U	%	3.5	3	5	6
Loss On Ignition	2610	U	%	4.9	--	--	10
Total BTEX	2760	U	mg/kg	< 0.010	6	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	< 0.10	1	--	--
TPH Total WAC	2670	U	mg/kg	< 10	500	--	--
Total (Of 17) PAH's	2800	N	mg/kg	8.1	100	--	--
pH	2010	U		8.0	--	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	0.065	--	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate mg/l	10:1 Eluate mg/kg	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1455	U	0.0041	0.041	0.5	2	25
Barium	1455	U	0.018	0.18	20	100	300
Cadmium	1455	U	< 0.00011	< 0.0011	0.04	1	5
Chromium	1455	U	< 0.0005	< 0.0050	0.5	10	70
Copper	1455	U	0.0031	0.031	2	50	100
Mercury	1455	U	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	U	0.0084	0.084	0.5	10	30
Nickel	1455	U	0.0011	0.011	0.4	10	40
Lead	1455	U	0.0008	0.0083	0.5	10	50
Antimony	1455	U	0.0074	0.074	0.06	0.7	5
Selenium	1455	U	0.0025	0.025	0.1	0.5	7
Zinc	1455	U	0.004	0.042	4	50	200
Chloride	1220	U	1.9	19	800	15000	25000
Fluoride	1220	U	0.22	2.2	10	150	500
Sulphate	1220	U	33	330	1000	20000	50000
Total Dissolved Solids	1020	N	130	1300	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.30	1	-	-
Dissolved Organic Carbon	1610	U	3.6	< 50	500	800	1000

Solid Information

Dry mass of test portion/kg	0.090
Moisture (%)	12

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Results - Single Stage WAC

Project: 22-1762 Richmond Road Phase 2

Project: 22-49100 Richmond Road Phase 2					Landfill Waste Acceptance Criteria		
Chemtest Job No: 22-49100					Limits		
Chemtest Sample ID: 1569218					Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Sample Ref: 2							
Sample ID:							
Sample Location: IT01							
Top Depth(m): 1.5							
Bottom Depth(m):							
Sampling Date: 22-Dec-2022							
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	U	%	3.4	3	5	6
Loss On Ignition	2610	U	%	15	--	--	10
Total BTEX	2760	U	mg/kg	< 0.010	6	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	< 0.10	1	--	--
TPH Total WAC	2670	U	mg/kg	< 10	500	--	--
Total (Of 17) PAH's	2800	N	mg/kg	4.5	100	--	--
pH	2010	U		7.6	--	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	0.092	--	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate mg/l	10:1 Eluate mg/kg	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1455	U	0.0053	0.052	0.5	2	25
Barium	1455	U	0.039	0.39	20	100	300
Cadmium	1455	U	< 0.00011	< 0.0011	0.04	1	5
Chromium	1455	U	< 0.0005	< 0.0050	0.5	10	70
Copper	1455	U	0.0016	0.016	2	50	100
Mercury	1455	U	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	U	0.025	0.25	0.5	10	30
Nickel	1455	U	0.0016	0.016	0.4	10	40
Lead	1455	U	0.0022	0.022	0.5	10	50
Antimony	1455	U	0.0065	0.064	0.06	0.7	5
Selenium	1455	U	0.0021	0.020	0.1	0.5	7
Zinc	1455	U	0.003	0.029	4	50	200
Chloride	1220	U	< 1.0	< 10	800	15000	25000
Fluoride	1220	U	0.20	2.0	10	150	500
Sulphate	1220	U	50	500	1000	20000	50000
Total Dissolved Solids	1020	N	240	2300	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.30	1	-	-
Dissolved Organic Carbon	1610	U	5.7	57	500	800	1000

Solid Information	
Dry mass of test portion/kg	0.090
Moisture (%)	29

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Results - Single Stage WAC

Project: 22-1762 Richmond Road Phase 2

Chemtest Job No: 22-49100					Landfill Waste Acceptance Criteria		
Chemtest Sample ID: 1569219					Limits		
Sample Ref: 1					Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Sample ID:							
Sample Location: IT02							
Top Depth(m): 0.5							
Bottom Depth(m):							
Sampling Date: 22-Dec-2022							
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	U	%	1.3	3	5	6
Loss On Ignition	2610	U	%	0.91	--	--	10
Total BTEX	2760	U	mg/kg	< 0.010	6	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	< 0.10	1	--	--
TPH Total WAC	2670	U	mg/kg	< 10	500	--	--
Total (Of 17) PAH's	2800	N	mg/kg	< 2.0	100	--	--
pH	2010	U		9.5	--	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	0.0050	--	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate mg/l	10:1 Eluate mg/kg	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1455	U	0.0022	0.022	0.5	2	25
Barium	1455	U	0.011	0.11	20	100	300
Cadmium	1455	U	< 0.00011	< 0.0011	0.04	1	5
Chromium	1455	U	0.0036	0.036	0.5	10	70
Copper	1455	U	0.0009	0.0094	2	50	100
Mercury	1455	U	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	U	0.0017	0.017	0.5	10	30
Nickel	1455	U	< 0.0005	< 0.0050	0.4	10	40
Lead	1455	U	< 0.0005	< 0.0050	0.5	10	50
Antimony	1455	U	0.0024	0.024	0.06	0.7	5
Selenium	1455	U	0.0012	0.012	0.1	0.5	7
Zinc	1455	U	< 0.003	< 0.025	4	50	200
Chloride	1220	U	3.8	38	800	15000	25000
Fluoride	1220	U	0.26	2.6	10	150	500
Sulphate	1220	U	11	110	1000	20000	50000
Total Dissolved Solids	1020	N	66	660	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.30	1	-	-
Dissolved Organic Carbon	1610	U	3.1	< 50	500	800	1000

Solid Information

Dry mass of test portion/kg	0.090
Moisture (%)	17

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Results - Single Stage WAC

Project: 22-1762 Richmond Road Phase 2

Chemtest Job No: 22-49100					Landfill Waste Acceptance Criteria		
Chemtest Sample ID: 1569220					Limits		
Sample Ref: 2					Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Sample ID:							
Sample Location: IT02							
Top Depth(m): 1.5							
Bottom Depth(m):							
Sampling Date: 22-Dec-2022							
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	U	%	1.8	3	5	6
Loss On Ignition	2610	U	%	19	--	--	10
Total BTEX	2760	U	mg/kg	< 0.010	6	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	< 0.10	1	--	--
TPH Total WAC	2670	U	mg/kg	< 10	500	--	--
Total (Of 17) PAH's	2800	N	mg/kg	< 2.0	100	--	--
pH	2010	U		9.0	--	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	0.18	--	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate mg/l	10:1 Eluate mg/kg	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1455	U	0.0044	0.043	0.5	2	25
Barium	1455	U	0.007	0.073	20	100	300
Cadmium	1455	U	< 0.00011	< 0.0011	0.04	1	5
Chromium	1455	U	0.0015	0.015	0.5	10	70
Copper	1455	U	0.0057	0.057	2	50	100
Mercury	1455	U	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	U	0.0079	0.080	0.5	10	30
Nickel	1455	U	0.0018	0.018	0.4	10	40
Lead	1455	U	0.0007	0.0066	0.5	10	50
Antimony	1455	U	0.0033	0.033	0.06	0.7	5
Selenium	1455	U	0.0022	0.022	0.1	0.5	7
Zinc	1455	U	0.006	0.058	4	50	200
Chloride	1220	U	6.3	63	800	15000	25000
Fluoride	1220	U	0.26	2.6	10	150	500
Sulphate	1220	U	11	110	1000	20000	50000
Total Dissolved Solids	1020	N	110	1100	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.30	1	-	-
Dissolved Organic Carbon	1610	U	4.8	< 50	500	800	1000

Solid Information

Dry mass of test portion/kg	0.090
Moisture (%)	27

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Results - Single Stage WAC

Project: 22-1762 Richmond Road Phase 2

Chemtest Job No: 22-49100					Landfill Waste Acceptance Criteria		
Chemtest Sample ID: 1569221					Limits		
Sample Ref: 1					Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Sample ID:							
Sample Location: TP01							
Top Depth(m): 0.5							
Bottom Depth(m):							
Sampling Date: 22-Dec-2022							
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	U	%	1.2	3	5	6
Loss On Ignition	2610	U	%	9.7	--	--	10
Total BTEX	2760	U	mg/kg	< 0.010	6	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	< 0.10	1	--	--
TPH Total WAC	2670	U	mg/kg	< 10	500	--	--
Total (Of 17) PAH's	2800	N	mg/kg	< 2.0	100	--	--
pH	2010	U		8.6	--	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	0.026	--	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate mg/l	10:1 Eluate mg/kg	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1455	U	0.0038	0.038	0.5	2	25
Barium	1455	U	0.005	0.052	20	100	300
Cadmium	1455	U	< 0.00011	< 0.0011	0.04	1	5
Chromium	1455	U	0.0066	0.066	0.5	10	70
Copper	1455	U	0.0012	0.012	2	50	100
Mercury	1455	U	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	U	0.0036	0.036	0.5	10	30
Nickel	1455	U	< 0.0005	< 0.0050	0.4	10	40
Lead	1455	U	< 0.0005	< 0.0050	0.5	10	50
Antimony	1455	U	0.0017	0.018	0.06	0.7	5
Selenium	1455	U	0.0015	0.015	0.1	0.5	7
Zinc	1455	U	< 0.003	< 0.025	4	50	200
Chloride	1220	U	< 1.0	< 10	800	15000	25000
Fluoride	1220	U	0.12	1.2	10	150	500
Sulphate	1220	U	4.5	45	1000	20000	50000
Total Dissolved Solids	1020	N	52	520	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.30	1	-	-
Dissolved Organic Carbon	1610	U	3.2	< 50	500	800	1000

Solid Information

Dry mass of test portion/kg	0.090
Moisture (%)	9.5

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Results - Single Stage WAC

Project: 22-1762 Richmond Road Phase 2

Chemtest Job No: 22-49100					Landfill Waste Acceptance Criteria		
Chemtest Sample ID: 1569222					Limits		
Sample Ref: 2					Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Sample ID:							
Sample Location: TP01							
Top Depth(m): 1.5							
Bottom Depth(m):							
Sampling Date: 22-Dec-2022							
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	U	%	3.3	3	5	6
Loss On Ignition	2610	U	%	3.8	--	--	10
Total BTEX	2760	U	mg/kg	< 0.010	6	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	< 0.10	1	--	--
TPH Total WAC	2670	U	mg/kg	< 10	500	--	--
Total (Of 17) PAH's	2800	N	mg/kg	< 2.0	100	--	--
pH	2010	U		8.2	--	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	0.025	--	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate mg/l	10:1 Eluate mg/kg	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1455	U	0.0012	0.012	0.5	2	25
Barium	1455	U	0.008	0.079	20	100	300
Cadmium	1455	U	< 0.00011	< 0.0011	0.04	1	5
Chromium	1455	U	< 0.0005	< 0.0050	0.5	10	70
Copper	1455	U	0.0011	0.011	2	50	100
Mercury	1455	U	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	U	0.013	0.13	0.5	10	30
Nickel	1455	U	0.0007	0.0065	0.4	10	40
Lead	1455	U	< 0.0005	< 0.0050	0.5	10	50
Antimony	1455	U	0.0012	0.012	0.06	0.7	5
Selenium	1455	U	0.0010	0.0098	0.1	0.5	7
Zinc	1455	U	< 0.003	< 0.025	4	50	200
Chloride	1220	U	< 1.0	< 10	800	15000	25000
Fluoride	1220	U	0.23	2.3	10	150	500
Sulphate	1220	U	1.6	16	1000	20000	50000
Total Dissolved Solids	1020	N	110	1100	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.30	1	-	-
Dissolved Organic Carbon	1610	U	2.8	< 50	500	800	1000

Solid Information

Dry mass of test portion/kg	0.090
Moisture (%)	19

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Results - Single Stage WAC

Project: 22-1762 Richmond Road Phase 2

Chemtest Job No: 22-49100					Landfill Waste Acceptance Criteria		
Chemtest Sample ID: 1569223					Limits		
Sample Ref: 1					Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Sample ID:							
Sample Location: TP02							
Top Depth(m): 0.5							
Bottom Depth(m):							
Sampling Date: 22-Dec-2022							
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	U	%	0.91	3	5	6
Loss On Ignition	2610	U	%	4.3	--	--	10
Total BTEX	2760	U	mg/kg	< 0.010	6	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	< 0.10	1	--	--
TPH Total WAC	2670	U	mg/kg	< 10	500	--	--
Total (Of 17) PAH's	2800	N	mg/kg	< 2.0	100	--	--
pH	2010	U		8.3	--	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	0.029	--	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate mg/l	10:1 Eluate mg/kg	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1455	U	0.031	0.30	0.5	2	25
Barium	1455	U	0.032	0.32	20	100	300
Cadmium	1455	U	< 0.00011	< 0.0011	0.04	1	5
Chromium	1455	U	0.047	0.47	0.5	10	70
Copper	1455	U	0.0011	0.011	2	50	100
Mercury	1455	U	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	U	0.0012	0.012	0.5	10	30
Nickel	1455	U	< 0.0005	< 0.0050	0.4	10	40
Lead	1455	U	0.0037	0.037	0.5	10	50
Antimony	1455	U	0.0015	0.015	0.06	0.7	5
Selenium	1455	U	0.0010	0.0098	0.1	0.5	7
Zinc	1455	U	0.007	0.073	4	50	200
Chloride	1220	U	< 1.0	< 10	800	15000	25000
Fluoride	1220	U	0.42	4.2	10	150	500
Sulphate	1220	U	6.4	64	1000	20000	50000
Total Dissolved Solids	1020	N	65	650	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.30	1	-	-
Dissolved Organic Carbon	1610	U	2.9	< 50	500	800	1000

Solid Information

Dry mass of test portion/kg	0.090
Moisture (%)	17

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Test Methods

SOP	Title	Parameters included	Method summary
1020	Electrical Conductivity and Total Dissolved Solids (TDS) in Waters	Electrical Conductivity and Total Dissolved Solids (TDS) in Waters	Conductivity Meter
1220	Anions, Alkalinity & Ammonium in Waters	Fluoride; Chloride; Nitrite; Nitrate; Total; Oxidisable Nitrogen (TON); Sulfate; Phosphate; Alkalinity; Ammonium	Automated colorimetric analysis using 'Aquakem 600' Discrete Analyser.
1455	Metals in Waters by ICP-MS	Metals, including: Antimony; Arsenic; Barium; Beryllium; Boron; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Tin; Vanadium; Zinc	Filtration of samples followed by direct determination by inductively coupled plasma mass spectrometry (ICP-MS).
1610	Total/Dissolved Organic Carbon in Waters	Organic Carbon	TOC Analyser using Catalytic Oxidation
1920	Phenols in Waters by HPLC	Phenolic compounds including: Phenol, Cresols, Xylenols, Trimethylphenols Note: Chlorophenols are excluded.	Determination by High Performance Liquid Chromatography (HPLC) using electrochemical detection.
2010	pH Value of Soils	pH	pH Meter
2015	Acid Neutralisation Capacity	Acid Reserve	Titration
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2040	Soil Description(Requirement of MCERTS)	Soil description	As received soil is described based upon BS5930
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2455	Acid Soluble Metals in Soils	Metals, including: Arsenic; Barium; Beryllium; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Vanadium; Zinc	Acid digestion followed by determination of metals in extract by ICP-MS.
2490	Hexavalent Chromium in Soils	Chromium [VI]	Soil extracts are prepared by extracting dried and ground soil samples into boiling water. Chromium [VI] is determined by 'Aquakem 600' Discrete Analyser using 1,5-diphenylcarbazide.
2610	Loss on Ignition	loss on ignition (LOI)	Determination of the proportion by mass that is lost from a soil by ignition at 550°C.
2625	Total Organic Carbon in Soils	Total organic Carbon (TOC)	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2670	Total Petroleum Hydrocarbons (TPH) in Soils by GC-FID	TPH (C6–C40); optional carbon banding, e.g. 3-band – GRO, DRO & LRO*TPH C8–C40	Dichloromethane extraction / GC-FID
2680	TPH A/A Split	Aliphatics: >C5–C6, >C6–C8,>C8–C10, >C10–C12, >C12–C16, >C16–C21, >C21–C35, >C35– C44Aromatics: >C5–C7, >C7–C8, >C8– C10, >C10–C12, >C12–C16, >C16– C21, >C21– C35, >C35– C44	Dichloromethane extraction / GCxGC FID detection
2760	Volatile Organic Compounds (VOCs) in Soils by Headspace GC-MS	Volatile organic compounds, including BTEX and halogenated Aliphatic/Aromatics.(cf. USEPA Method 8260)*please refer to UKAS schedule	Automated headspace gas chromatographic (GC) analysis of a soil sample, as received, with mass spectrometric (MS) detection of volatile organic compounds.
2800	Speciated Polynuclear Aromatic Hydrocarbons (PAH) in Soil by GC-MS	Acenaphthene*; Acenaphthylene; Anthracene*; Benzo[a]Anthracene*; Benzo[a]Pyrene*; Benzo[b]Fluoranthene*; Benzo[ghi]Perylene*; Benzo[k]Fluoranthene*; Chrysene*; Dibenzo[ah]Anthracene; Fluoranthene*; Fluorene*; Indeno[123cd]Pyrene*; Naphthalene*; Phenanthrene*; Pyrene*	Dichloromethane extraction / GC-MS

Test Methods

SOP	Title	Parameters included	Method summary
2815	Polychlorinated Biphenyls (PCB) ICES7Congeners in Soils by GC-MS	ICES7 PCB congeners	Acetone/Hexane extraction / GC-MS
640	Characterisation of Waste (Leaching C10)	Waste material including soil, sludges and granular waste	ComplianceTest for Leaching of Granular Waste Material and Sludge

Report Information

Key

U	UKAS accredited
M	MCERTS and UKAS accredited
N	Unaccredited
S	This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
SN	This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
T	This analysis has been subcontracted to an unaccredited laboratory
I/S	Insufficient Sample
U/S	Unsuitable Sample
N/E	not evaluated
<	"less than"
>	"greater than"
SOP	Standard operating procedure
LOD	Limit of detection

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

Sample Deviation Codes

- A - Date of sampling not supplied
- B - Sample age exceeds stability time (sampling to extraction)
- C - Sample not received in appropriate containers
- D - Broken Container
- E - Insufficient Sample (Applies to LOI in Trommel Fines Only)

Sample Retention and Disposal

All soil samples will be retained for a period of 30 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to:

customerservices@chemtest.com



CAUSEWAY
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APPENDIX G

WASTE CLASSIFICATION REPORT



Waste Classification Report

HazWasteOnline™ classifies waste as either **hazardous** or **non-hazardous** based on its chemical composition, related legislation and the rules and data defined in the current UK or EU technical guidance (Appendix C) (note that HP 9 Infectious is not assessed). It is the responsibility of the classifier named below to:

- understand the origin of the waste
- select the correct List of Waste code(s)
- confirm that the list of determinands, results and sampling plan are fit for purpose
- select and justify the chosen metal species (Appendix B)
- correctly apply moisture correction and other available corrections
- add the meta data for their user-defined substances (Appendix A)
- check that the classification engine is suitable with respect to the national destination of the waste (Appendix C)



ACPCC-JVDA5-16WD8

To aid the reviewer, the laboratory results, assumptions and justifications managed by the classifier are highlighted in **pale yellow**.

Job name

22-1762 Richmond Road Phase 2

Description/Comments

Waste classification on samples recovered from site in December 2022.

Project

22-1762

Site

Richmond Road Phase 2

Classified by

Name: **Sean Ross**
 Date: **20 Jan 2023 10:40 GMT**
 Telephone:
 Company: **Causeway Geotech Ltd**
Unit 1 Fingal House, Stephenstown
Industrial Estate,
Balbriggan
K32 VR66

HazWasteOnline™ provides a two day, hazardous waste classification course that covers the use of the software and both basic and advanced waste classification techniques. Certification has to be renewed every 3 years.

HazWasteOnline™ Certification:

Course
 Hazardous Waste Classification

Date
 50% complete

Purpose of classification

2 - Material Characterisation

Address of the waste

Richmond Road, Dublin.

Post Code **N/A**

SIC for the process giving rise to the waste

Description of industry/producer giving rise to the waste

Redevelopment of industrial site into a residential development.

Description of the specific process, sub-process and/or activity that created the waste

Waste created during redevelopment of the site.

Description of the waste

Generally reworked soft to firm brown sandy gravelly clay with construction waste (e.g. concrete, brick and scrap metal fragments).

Job summary

#	Sample name	Depth [m]	Classification Result	Hazard properties	WAC Results		Page
					Inert	Non Haz	
1	IT01-1-22/12/2022-0.5	0.5	Non Hazardous		Fail	Pass	3
2	IT01-2-22/12/2022-1.5	1.5	Non Hazardous		Fail	Pass	6
3	IT02-1-22/12/2022-0.5	0.5	Non Hazardous		Pass	Pass	9
4	IT02-2-22/12/2022-1.5	1.5	Non Hazardous		Pass	Pass	12
5	TP01-1-22/12/2022-0.5	0.5	Non Hazardous		Pass	Pass	15
6	TP01-2-22/12/2022-1.5	1.5	Non Hazardous		Fail	Pass	18
7	TP02-1-22/12/2022-0.5	0.5	Non Hazardous		Pass	Pass	21

Related documents

#	Name	Description
1	HWOL_22-49100-20230118 143343.hwol	Eurofins Chemtest .hwol file used to populate the Job
2	Example waste stream template for contaminated soils	waste stream template used to create this Job

WAC results

WAC Settings: samples in this Job do not constitute a single population.

WAC limits used to evaluate the samples in this Job: "Ireland"

The WAC used in this report are the WAC defined for the inert and non-hazardous classes of landfill in the Republic of Ireland. You should check the actual acceptance criteria when the disposal site is identified as they may differ from the generic WAC used in this report.

Report

Created by: Sean Ross

Created date: 20 Jan 2023 10:40 GMT

Appendices	Page
Appendix A: Classifier defined and non EU CLP determinands	24
Appendix B: Rationale for selection of metal species	25
Appendix C: Version	26

Classification of sample: IT01-1-22/12/2022-0.5

✔ **Non Hazardous Waste**
Classified as **17 05 04**
in the List of Waste

Sample details

Sample name:	LoW Code:
IT01-1-22/12/2022-0.5	Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Sample Depth:	Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)
0.5 m	
Moisture content:	
12%	
(wet weight correction)	

Hazard properties

None identified

Determinands

Moisture content: 12% Wet Weight Moisture Correction applied (MC)

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
1	antimony { antimony trioxide }				<2 mg/kg	1.197	<2.394 mg/kg	<0.000239 %		<LOD
	051-005-00-X	215-175-0	1309-64-4							
2	arsenic { arsenic trioxide }				8.9 mg/kg	1.32	10.341 mg/kg	0.00103 %	✓	
	033-003-00-0	215-481-4	1327-53-3							
3	cadmium { cadmium oxide }				0.54 mg/kg	1.142	0.543 mg/kg	0.0000543 %	✓	
	048-002-00-0	215-146-2	1306-19-0							
4	chromium in chromium(III) compounds { chromium(III) oxide (worst case) }				5.3 mg/kg	1.462	6.817 mg/kg	0.000682 %	✓	
		215-160-9	1308-38-9							
5	chromium in chromium(VI) compounds { chromium(VI) compounds, with the exception of barium chromate and of compounds specified elsewhere in this Annex }				<0.5 mg/kg	2.27	<1.135 mg/kg	<0.000113 %		<LOD
	024-017-00-8									
6	copper { dicopper oxide; copper (I) oxide }				41 mg/kg	1.126	40.622 mg/kg	0.00406 %	✓	
	029-002-00-X	215-270-7	1317-39-1							
7	lead { lead chromate }			1	150 mg/kg	1.56	205.896 mg/kg	0.0132 %	✓	
	082-004-00-2	231-846-0	7758-97-6							
8	mercury { mercury dichloride }				0.31 mg/kg	1.353	0.369 mg/kg	0.0000369 %	✓	
	080-010-00-X	231-299-8	7487-94-7							
9	molybdenum { molybdenum(VI) oxide }				2.3 mg/kg	1.5	3.036 mg/kg	0.000304 %	✓	
	042-001-00-9	215-204-7	1313-27-5							
10	nickel { nickel chromate }				11 mg/kg	2.976	28.81 mg/kg	0.00288 %	✓	
	028-035-00-7	238-766-5	14721-18-7							
11	selenium { nickel selenate }				0.66 mg/kg	2.554	1.483 mg/kg	0.000148 %	✓	
	028-031-00-5	239-125-2	15060-62-5							
12	zinc { zinc chromate }				130 mg/kg	2.774	317.362 mg/kg	0.0317 %	✓	
	024-007-00-3	236-878-9	13530-65-9							
13	tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	603-181-00-X	216-653-1	1634-04-4							
14	benzene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	601-020-00-8	200-753-7	71-43-2							
15	toluene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	601-021-00-3	203-625-9	108-88-3							

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
16	ethylbenzene 601-023-00-4	202-849-4	100-41-4		<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
17	xylene 601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<0.002 mg/kg		<0.002 mg/kg	<0.0000002 %		<LOD
18	naphthalene 601-052-00-2	202-049-5	91-20-3		0.25 mg/kg		0.22 mg/kg	0.000022 %	✓	
19	acenaphthylene 205-917-1	208-96-8			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
20	acenaphthene 201-469-6	83-32-9			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
21	fluorene 201-695-5	86-73-7			0.15 mg/kg		0.132 mg/kg	0.0000132 %	✓	
22	phenanthrene 201-581-5	85-01-8			1 mg/kg		0.88 mg/kg	0.000088 %	✓	
23	anthracene 204-371-1	120-12-7			0.22 mg/kg		0.194 mg/kg	0.0000194 %	✓	
24	fluoranthene 205-912-4	206-44-0			1.3 mg/kg		1.144 mg/kg	0.000114 %	✓	
25	pyrene 204-927-3	129-00-0			1.1 mg/kg		0.968 mg/kg	0.0000968 %	✓	
26	benzo[a]anthracene 601-033-00-9	200-280-6	56-55-3		0.65 mg/kg		0.572 mg/kg	0.0000572 %	✓	
27	chrysene 601-048-00-0	205-923-4	218-01-9		0.6 mg/kg		0.528 mg/kg	0.0000528 %	✓	
28	benzo[b]fluoranthene 601-034-00-4	205-911-9	205-99-2		0.91 mg/kg		0.801 mg/kg	0.0000801 %	✓	
29	benzo[k]fluoranthene 601-036-00-5	205-916-6	207-08-9		0.25 mg/kg		0.22 mg/kg	0.000022 %	✓	
30	benzo[a]pyrene; benzo[def]chrysene 601-032-00-3	200-028-5	50-32-8		0.65 mg/kg		0.572 mg/kg	0.0000572 %	✓	
31	indeno[123-cd]pyrene 205-893-2	193-39-5			0.46 mg/kg		0.405 mg/kg	0.0000405 %	✓	
32	dibenz[a,h]anthracene 601-041-00-2	200-181-8	53-70-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
33	benzo[ghi]perylene 205-883-8	191-24-2			0.55 mg/kg		0.484 mg/kg	0.0000484 %	✓	
34	polychlorobiphenyls; PCB 602-039-00-4	215-648-1	1336-36-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
35	barium { barium sulphide } 016-002-00-X	244-214-4	21109-95-5		77 mg/kg	1.233	83.582 mg/kg	0.00836 %	✓	
36	coronene 205-881-7	191-07-1			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
Total:								0.0636 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
•	Determinand defined or amended by HazWasteOnline (see Appendix A)
	Speciated Determinand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
ND	Not detected
CLP: Note 1	Only the metal concentration has been used for classification

WAC results for sample: IT01-1-22/12/2022-0.5

WAC Settings: samples in this Job do not constitute a single population.

WAC limits used to evaluate this sample: "Ireland"

The WAC used in this report are the WAC defined for the inert and non-hazardous classes of landfill in the Republic of Ireland. You should check the actual acceptance criteria when the disposal site is identified as they may differ from the generic WAC used in this report.

The sample FAILS the Inert (Inert waste landfill) criteria.

The sample PASSES the Non Haz (Non hazardous waste landfill) criteria.

WAC Determinands

Solid Waste Analysis				Landfill Waste Acceptance Criteria Limits	
#	Determinand		User entered data	Inert waste landfill	Non hazardous waste landfill
1	TOC (total organic carbon)	%	3.5	3	5
2	LOI (loss on ignition)	%	4.9	-	-
3	BTEX (benzene, toluene, ethylbenzene and xylenes)	mg/kg	<0.01	6	-
4	PCBs (polychlorinated biphenyls, 7 congeners)	mg/kg	<0.1	1	-
5	Mineral oil (C10 to C40)	mg/kg	<10	500	-
6	PAHs (polycyclic aromatic hydrocarbons)	mg/kg	8.1	100	-
7	pH	pH	8	-	>6
8	ANC (acid neutralisation capacity)	mol/kg	0.065	-	-
Eluate Analysis 10:1					
9	arsenic	mg/kg	0.041	0.5	2
10	barium	mg/kg	0.18	20	100
11	cadmium	mg/kg	<0.0011	0.04	1
12	chromium	mg/kg	<0.005	0.5	10
13	copper	mg/kg	0.031	2	50
14	mercury	mg/kg	<0.0005	0.01	0.2
15	molybdenum	mg/kg	0.084	0.5	10
16	nickel	mg/kg	0.011	0.4	10
17	lead	mg/kg	0.0083	0.5	10
18	antimony	mg/kg	0.074	0.06	0.7
19	selenium	mg/kg	0.025	0.1	0.5
20	zinc	mg/kg	0.042	4	50
21	chloride	mg/kg	19	800	15,000
22	fluoride	mg/kg	2.2	10	150
23	sulphate	mg/kg	330	1,000	20,000
24	phenol index	mg/kg	<0.3	1	-
25	DOC (dissolved organic carbon)	mg/kg	<50	500	800
26	TDS (total dissolved solids)	mg/kg	1300	4,000	60,000

Key

	User supplied data
	Inert WAC criteria fail

Classification of sample: IT01-2-22/12/2022-1.5

 **Non Hazardous Waste**
 Classified as **17 05 04**
 in the List of Waste

Sample details

Sample name:	LoW Code:
IT01-2-22/12/2022-1.5	Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Sample Depth:	Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)
1.5 m	
Moisture content:	
29%	
(wet weight correction)	

Hazard properties

None identified

Determinands

Moisture content: 29% Wet Weight Moisture Correction applied (MC)

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number								
1	antimony { antimony trioxide }				<2 mg/kg	1.197	<2.394 mg/kg	<0.000239 %			<LOD
	051-005-00-X	215-175-0	1309-64-4								
2	arsenic { arsenic trioxide }				7.2 mg/kg	1.32	6.75 mg/kg	0.000675 %		✓	
	033-003-00-0	215-481-4	1327-53-3								
3	cadmium { cadmium oxide }				0.78 mg/kg	1.142	0.633 mg/kg	0.0000633 %		✓	
	048-002-00-0	215-146-2	1306-19-0								
4	chromium in chromium(III) compounds { chromium(III) oxide (worst case) }				8.9 mg/kg	1.462	9.236 mg/kg	0.000924 %		✓	
		215-160-9	1308-38-9								
5	chromium in chromium(VI) compounds { chromium(VI) compounds, with the exception of barium chromate and of compounds specified elsewhere in this Annex }				<0.5 mg/kg	2.27	<1.135 mg/kg	<0.000113 %			<LOD
	024-017-00-8										
6	copper { dicopper oxide; copper (I) oxide }				21 mg/kg	1.126	16.787 mg/kg	0.00168 %		✓	
	029-002-00-X	215-270-7	1317-39-1								
7	lead { lead chromate }			1	55 mg/kg	1.56	60.911 mg/kg	0.0039 %		✓	
	082-004-00-2	231-846-0	7758-97-6								
8	mercury { mercury dichloride }				0.22 mg/kg	1.353	0.211 mg/kg	0.0000211 %		✓	
	080-010-00-X	231-299-8	7487-94-7								
9	molybdenum { molybdenum(VI) oxide }				1.9 mg/kg	1.5	2.024 mg/kg	0.000202 %		✓	
	042-001-00-9	215-204-7	1313-27-5								
10	nickel { nickel chromate }				22 mg/kg	2.976	46.489 mg/kg	0.00465 %		✓	
	028-035-00-7	238-766-5	14721-18-7								
11	selenium { nickel selenate }				0.69 mg/kg	2.554	1.251 mg/kg	0.000125 %		✓	
	028-031-00-5	239-125-2	15060-62-5								
12	zinc { zinc chromate }				68 mg/kg	2.774	133.936 mg/kg	0.0134 %		✓	
	024-007-00-3	236-878-9	13530-65-9								
13	tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
	603-181-00-X	216-653-1	1634-04-4								
14	benzene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
	601-020-00-8	200-753-7	71-43-2								
15	toluene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
	601-021-00-3	203-625-9	108-88-3								

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
16	ethylbenzene 601-023-00-4	202-849-4	100-41-4		<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
17	xylene 601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<0.002 mg/kg		<0.002 mg/kg	<0.0000002 %		<LOD
18	naphthalene 601-052-00-2	202-049-5	91-20-3		0.11 mg/kg		0.0781 mg/kg	0.00000781 %	✓	
19	acenaphthylene 205-917-1	208-96-8			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
20	acenaphthene 201-469-6	83-32-9			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
21	fluorene 201-695-5	86-73-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
22	phenanthrene 201-581-5	85-01-8			0.42 mg/kg		0.298 mg/kg	0.0000298 %	✓	
23	anthracene 204-371-1	120-12-7			0.11 mg/kg		0.0781 mg/kg	0.00000781 %	✓	
24	fluoranthene 205-912-4	206-44-0			0.66 mg/kg		0.469 mg/kg	0.0000469 %	✓	
25	pyrene 204-927-3	129-00-0			0.64 mg/kg		0.454 mg/kg	0.0000454 %	✓	
26	benzo[a]anthracene 601-033-00-9	200-280-6	56-55-3		0.39 mg/kg		0.277 mg/kg	0.0000277 %	✓	
27	chrysene 601-048-00-0	205-923-4	218-01-9		0.32 mg/kg		0.227 mg/kg	0.0000227 %	✓	
28	benzo[b]fluoranthene 601-034-00-4	205-911-9	205-99-2		0.59 mg/kg		0.419 mg/kg	0.0000419 %	✓	
29	benzo[k]fluoranthene 601-036-00-5	205-916-6	207-08-9		0.24 mg/kg		0.17 mg/kg	0.000017 %	✓	
30	benzo[a]pyrene; benzo[def]chrysene 601-032-00-3	200-028-5	50-32-8		0.41 mg/kg		0.291 mg/kg	0.0000291 %	✓	
31	indeno[123-cd]pyrene 205-893-2	193-39-5			0.35 mg/kg		0.248 mg/kg	0.0000248 %	✓	
32	dibenz[a,h]anthracene 601-041-00-2	200-181-8	53-70-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
33	benzo[ghi]perylene 205-883-8	191-24-2			0.3 mg/kg		0.213 mg/kg	0.0000213 %	✓	
34	polychlorobiphenyls; PCB 602-039-00-4	215-648-1	1336-36-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
35	barium { barium sulphide } 016-002-00-X	244-214-4	21109-95-5		55 mg/kg	1.233	48.168 mg/kg	0.00482 %	✓	
36	coronene 205-881-7	191-07-1			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
Total:								0.0312 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
•	Determinand defined or amended by HazWasteOnline (see Appendix A)
•	Speciated Determinand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
ND	Not detected
CLP: Note 1	Only the metal concentration has been used for classification

WAC results for sample: IT01-2-22/12/2022-1.5

WAC Settings: samples in this Job do not constitute a single population.

WAC limits used to evaluate this sample: "Ireland"

The WAC used in this report are the WAC defined for the inert and non-hazardous classes of landfill in the Republic of Ireland. You should check the actual acceptance criteria when the disposal site is identified as they may differ from the generic WAC used in this report.

The sample FAILS the Inert (Inert waste landfill) criteria.

The sample PASSES the Non Haz (Non hazardous waste landfill) criteria.

WAC Determinands

Solid Waste Analysis				Landfill Waste Acceptance Criteria Limits	
#	Determinand		User entered data	Inert waste landfill	Non hazardous waste landfill
1	TOC (total organic carbon)	%	3.4	3	5
2	LOI (loss on ignition)	%	15	-	-
3	BTEX (benzene, toluene, ethylbenzene and xylenes)	mg/kg	<0.01	6	-
4	PCBs (polychlorinated biphenyls, 7 congeners)	mg/kg	<0.1	1	-
5	Mineral oil (C10 to C40)	mg/kg	<10	500	-
6	PAHs (polycyclic aromatic hydrocarbons)	mg/kg	4.5	100	-
7	pH	pH	7.6	-	>6
8	ANC (acid neutralisation capacity)	mol/kg	0.092	-	-
Eluate Analysis 10:1					
9	arsenic	mg/kg	0.052	0.5	2
10	barium	mg/kg	0.39	20	100
11	cadmium	mg/kg	<0.0011	0.04	1
12	chromium	mg/kg	<0.005	0.5	10
13	copper	mg/kg	0.016	2	50
14	mercury	mg/kg	<0.0005	0.01	0.2
15	molybdenum	mg/kg	0.25	0.5	10
16	nickel	mg/kg	0.016	0.4	10
17	lead	mg/kg	0.022	0.5	10
18	antimony	mg/kg	0.064	0.06	0.7
19	selenium	mg/kg	0.02	0.1	0.5
20	zinc	mg/kg	0.029	4	50
21	chloride	mg/kg	<10	800	15,000
22	fluoride	mg/kg	2	10	150
23	sulphate	mg/kg	500	1,000	20,000
24	phenol index	mg/kg	<0.3	1	-
25	DOC (dissolved organic carbon)	mg/kg	57	500	800
26	TDS (total dissolved solids)	mg/kg	2300	4,000	60,000

Key

	User supplied data
	Inert WAC criteria fail

Classification of sample: IT02-1-22/12/2022-0.5

✔ **Non Hazardous Waste**
Classified as **17 05 04**
in the List of Waste

Sample details

Sample name:	LoW Code:
IT02-1-22/12/2022-0.5	Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Sample Depth:	Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)
0.5 m	
Moisture content:	
17%	
(wet weight correction)	

Hazard properties

None identified

Determinands

Moisture content: 17% Wet Weight Moisture Correction applied (MC)

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
1	antimony { antimony trioxide }				<2 mg/kg	1.197	<2.394 mg/kg	<0.000239 %		<LOD
	051-005-00-X	215-175-0	1309-64-4							
2	arsenic { arsenic trioxide }				8.1 mg/kg	1.32	8.877 mg/kg	0.000888 %	✓	
	033-003-00-0	215-481-4	1327-53-3							
3	cadmium { cadmium oxide }				0.69 mg/kg	1.142	0.654 mg/kg	0.0000654 %	✓	
	048-002-00-0	215-146-2	1306-19-0							
4	chromium in chromium(III) compounds { chromium(III) oxide (worst case) }				8.7 mg/kg	1.462	10.554 mg/kg	0.00106 %	✓	
		215-160-9	1308-38-9							
5	chromium in chromium(VI) compounds { chromium(VI) compounds, with the exception of barium chromate and of compounds specified elsewhere in this Annex }				<0.5 mg/kg	2.27	<1.135 mg/kg	<0.000113 %		<LOD
	024-017-00-8									
6	copper { dicopper oxide; copper (I) oxide }				19 mg/kg	1.126	17.755 mg/kg	0.00178 %	✓	
	029-002-00-X	215-270-7	1317-39-1							
7	lead { lead chromate }			1	120 mg/kg	1.56	155.358 mg/kg	0.00996 %	✓	
	082-004-00-2	231-846-0	7758-97-6							
8	mercury { mercury dichloride }				1.1 mg/kg	1.353	1.236 mg/kg	0.000124 %	✓	
	080-010-00-X	231-299-8	7487-94-7							
9	molybdenum { molybdenum(VI) oxide }				1.3 mg/kg	1.5	1.619 mg/kg	0.000162 %	✓	
	042-001-00-9	215-204-7	1313-27-5							
10	nickel { nickel chromate }				18 mg/kg	2.976	44.465 mg/kg	0.00445 %	✓	
	028-035-00-7	238-766-5	14721-18-7							
11	selenium { nickel selenate }				0.53 mg/kg	2.554	1.123 mg/kg	0.000112 %	✓	
	028-031-00-5	239-125-2	15060-62-5							
12	zinc { zinc chromate }				130 mg/kg	2.774	299.33 mg/kg	0.0299 %	✓	
	024-007-00-3	236-878-9	13530-65-9							
13	tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	603-181-00-X	216-653-1	1634-04-4							
14	benzene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	601-020-00-8	200-753-7	71-43-2							
15	toluene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	601-021-00-3	203-625-9	108-88-3							

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
16	ethylbenzene 601-023-00-4	202-849-4	100-41-4		<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
17	xylene 601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<0.002 mg/kg		<0.002 mg/kg	<0.0000002 %		<LOD
18	naphthalene 601-052-00-2	202-049-5	91-20-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
19	acenaphthylene 205-917-1	208-96-8			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
20	acenaphthene 201-469-6	83-32-9			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
21	fluorene 201-695-5	86-73-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
22	phenanthrene 201-581-5	85-01-8			0.23 mg/kg		0.191 mg/kg	0.0000191 %	✓	
23	anthracene 204-371-1	120-12-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
24	fluoranthene 205-912-4	206-44-0			0.29 mg/kg		0.241 mg/kg	0.0000241 %	✓	
25	pyrene 204-927-3	129-00-0			0.24 mg/kg		0.199 mg/kg	0.0000199 %	✓	
26	benzo[a]anthracene 601-033-00-9	200-280-6	56-55-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
27	chrysene 601-048-00-0	205-923-4	218-01-9		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
28	benzo[b]fluoranthene 601-034-00-4	205-911-9	205-99-2		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
29	benzo[k]fluoranthene 601-036-00-5	205-916-6	207-08-9		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
30	benzo[a]pyrene; benzo[def]chrysene 601-032-00-3	200-028-5	50-32-8		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
31	indeno[123-cd]pyrene 205-893-2	193-39-5			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
32	dibenz[a,h]anthracene 601-041-00-2	200-181-8	53-70-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
33	benzo[ghi]perylene 205-883-8	191-24-2			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
34	polychlorobiphenyls; PCB 602-039-00-4	215-648-1	1336-36-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
35	barium { barium sulphide } 016-002-00-X	244-214-4	21109-95-5		220 mg/kg	1.233	225.236 mg/kg	0.0225 %	✓	
36	coronene 205-881-7	191-07-1			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
Total:								0.0716 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
●	Determinand defined or amended by HazWasteOnline (see Appendix A)
🧪	Speciated Determinand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
ND	Not detected
CLP: Note 1	Only the metal concentration has been used for classification

WAC results for sample: IT02-1-22/12/2022-0.5

WAC Settings: samples in this Job do not constitute a single population.

WAC limits used to evaluate this sample: "Ireland"

The WAC used in this report are the WAC defined for the inert and non-hazardous classes of landfill in the Republic of Ireland. You should check the actual acceptance criteria when the disposal site is identified as they may differ from the generic WAC used in this report.

The sample **PASSES** the Inert (Inert waste landfill) criteria.

The sample **PASSES** the Non Haz (Non hazardous waste landfill) criteria.

WAC Determinands

Solid Waste Analysis				Landfill Waste Acceptance Criteria Limits	
#	Determinand		User entered data	Inert waste landfill	Non hazardous waste landfill
1	TOC (total organic carbon)	%	1.3	3	5
2	LOI (loss on ignition)	%	0.91	-	-
3	BTEX (benzene, toluene, ethylbenzene and xylenes)	mg/kg	<0.01	6	-
4	PCBs (polychlorinated biphenyls, 7 congeners)	mg/kg	<0.1	1	-
5	Mineral oil (C10 to C40)	mg/kg	<10	500	-
6	PAHs (polycyclic aromatic hydrocarbons)	mg/kg	<2	100	-
7	pH	pH	9.5	-	>6
8	ANC (acid neutralisation capacity)	mol/kg	0.005	-	-
Eluate Analysis 10:1					
9	arsenic	mg/kg	0.022	0.5	2
10	barium	mg/kg	0.11	20	100
11	cadmium	mg/kg	<0.0011	0.04	1
12	chromium	mg/kg	0.036	0.5	10
13	copper	mg/kg	0.0094	2	50
14	mercury	mg/kg	<0.0005	0.01	0.2
15	molybdenum	mg/kg	0.017	0.5	10
16	nickel	mg/kg	<0.005	0.4	10
17	lead	mg/kg	<0.005	0.5	10
18	antimony	mg/kg	0.024	0.06	0.7
19	selenium	mg/kg	0.012	0.1	0.5
20	zinc	mg/kg	<0.025	4	50
21	chloride	mg/kg	38	800	15,000
22	fluoride	mg/kg	2.6	10	150
23	sulphate	mg/kg	110	1,000	20,000
24	phenol index	mg/kg	<0.3	1	-
25	DOC (dissolved organic carbon)	mg/kg	<50	500	800
26	TDS (total dissolved solids)	mg/kg	660	4,000	60,000

Key

User supplied data

Classification of sample: IT02-2-22/12/2022-1.5

 **Non Hazardous Waste**
 Classified as **17 05 04**
 in the List of Waste

Sample details

Sample name:	LoW Code:
IT02-2-22/12/2022-1.5	Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Sample Depth:	Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)
1.5 m	
Moisture content:	
27%	
(wet weight correction)	

Hazard properties

None identified

Determinands

Moisture content: 27% Wet Weight Moisture Correction applied (MC)

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number								
1	antimony { antimony trioxide }				<2 mg/kg	1.197	<2.394 mg/kg	<0.000239 %			<LOD
	051-005-00-X	215-175-0	1309-64-4								
2	arsenic { arsenic trioxide }				7.6 mg/kg	1.32	7.325 mg/kg	0.000733 %		✓	
	033-003-00-0	215-481-4	1327-53-3								
3	cadmium { cadmium oxide }				0.66 mg/kg	1.142	0.55 mg/kg	0.000055 %		✓	
	048-002-00-0	215-146-2	1306-19-0								
4	chromium in chromium(III) compounds { chromium(III) oxide (worst case) }				8.2 mg/kg	1.462	8.749 mg/kg	0.000875 %		✓	
		215-160-9	1308-38-9								
5	chromium in chromium(VI) compounds { chromium(VI) compounds, with the exception of barium chromate and of compounds specified elsewhere in this Annex }				<0.5 mg/kg	2.27	<1.135 mg/kg	<0.000113 %			<LOD
	024-017-00-8										
6	copper { dicopper oxide; copper (I) oxide }				17 mg/kg	1.126	13.972 mg/kg	0.0014 %		✓	
	029-002-00-X	215-270-7	1317-39-1								
7	lead { lead chromate }			1	60 mg/kg	1.56	68.32 mg/kg	0.00438 %		✓	
	082-004-00-2	231-846-0	7758-97-6								
8	mercury { mercury dichloride }				0.4 mg/kg	1.353	0.395 mg/kg	0.0000395 %		✓	
	080-010-00-X	231-299-8	7487-94-7								
9	molybdenum { molybdenum(VI) oxide }				1.2 mg/kg	1.5	1.314 mg/kg	0.000131 %		✓	
	042-001-00-9	215-204-7	1313-27-5								
10	nickel { nickel chromate }				17 mg/kg	2.976	36.935 mg/kg	0.00369 %		✓	
	028-035-00-7	238-766-5	14721-18-7								
11	selenium { nickel selenate }				0.49 mg/kg	2.554	0.914 mg/kg	0.0000914 %		✓	
	028-031-00-5	239-125-2	15060-62-5								
12	zinc { zinc chromate }				66 mg/kg	2.774	133.658 mg/kg	0.0134 %		✓	
	024-007-00-3	236-878-9	13530-65-9								
13	tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
	603-181-00-X	216-653-1	1634-04-4								
14	benzene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
	601-020-00-8	200-753-7	71-43-2								
15	toluene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
	601-021-00-3	203-625-9	108-88-3								

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
16	ethylbenzene 601-023-00-4	202-849-4	100-41-4		<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
17	xylene 601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<0.002 mg/kg		<0.002 mg/kg	<0.0000002 %		<LOD
18	naphthalene 601-052-00-2	202-049-5	91-20-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
19	acenaphthylene 205-917-1	208-96-8			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
20	acenaphthene 201-469-6	83-32-9			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
21	fluorene 201-695-5	86-73-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
22	phenanthrene 201-581-5	85-01-8			0.17 mg/kg		0.124 mg/kg	0.0000124 %	✓	
23	anthracene 204-371-1	120-12-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
24	fluoranthene 205-912-4	206-44-0			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
25	pyrene 204-927-3	129-00-0			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
26	benzo[a]anthracene 601-033-00-9	200-280-6	56-55-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
27	chrysene 601-048-00-0	205-923-4	218-01-9		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
28	benzo[b]fluoranthene 601-034-00-4	205-911-9	205-99-2		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
29	benzo[k]fluoranthene 601-036-00-5	205-916-6	207-08-9		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
30	benzo[a]pyrene; benzo[def]chrysene 601-032-00-3	200-028-5	50-32-8		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
31	indeno[123-cd]pyrene 205-893-2	193-39-5			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
32	dibenz[a,h]anthracene 601-041-00-2	200-181-8	53-70-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
33	benzo[ghi]perylene 205-883-8	191-24-2			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
34	polychlorobiphenyls; PCB 602-039-00-4	215-648-1	1336-36-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
35	barium { barium sulphide } 016-002-00-X	244-214-4	21109-95-5		59 mg/kg	1.233	53.127 mg/kg	0.00531 %	✓	
36	coronene 205-881-7	191-07-1			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
Total:								0.0306 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
•	Determinand defined or amended by HazWasteOnline (see Appendix A)
•	Speciated Determinand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
ND	Not detected
CLP: Note 1	Only the metal concentration has been used for classification

WAC results for sample: IT02-2-22/12/2022-1.5

WAC Settings: samples in this Job do not constitute a single population.

WAC limits used to evaluate this sample: "Ireland"

The WAC used in this report are the WAC defined for the inert and non-hazardous classes of landfill in the Republic of Ireland. You should check the actual acceptance criteria when the disposal site is identified as they may differ from the generic WAC used in this report.

The sample **PASSES** the Inert (Inert waste landfill) criteria.

The sample **PASSES** the Non Haz (Non hazardous waste landfill) criteria.

WAC Determinands

Solid Waste Analysis				Landfill Waste Acceptance Criteria Limits	
#	Determinand		User entered data	Inert waste landfill	Non hazardous waste landfill
1	TOC (total organic carbon)	%	1.8	3	5
2	LOI (loss on ignition)	%	19	-	-
3	BTEX (benzene, toluene, ethylbenzene and xylenes)	mg/kg	<0.01	6	-
4	PCBs (polychlorinated biphenyls, 7 congeners)	mg/kg	<0.1	1	-
5	Mineral oil (C10 to C40)	mg/kg	<10	500	-
6	PAHs (polycyclic aromatic hydrocarbons)	mg/kg	<2	100	-
7	pH	pH	9	-	>6
8	ANC (acid neutralisation capacity)	mol/kg	0.18	-	-
Eluate Analysis 10:1					
9	arsenic	mg/kg	0.043	0.5	2
10	barium	mg/kg	0.073	20	100
11	cadmium	mg/kg	<0.0011	0.04	1
12	chromium	mg/kg	0.015	0.5	10
13	copper	mg/kg	0.057	2	50
14	mercury	mg/kg	<0.0005	0.01	0.2
15	molybdenum	mg/kg	0.08	0.5	10
16	nickel	mg/kg	0.018	0.4	10
17	lead	mg/kg	0.0066	0.5	10
18	antimony	mg/kg	0.033	0.06	0.7
19	selenium	mg/kg	0.022	0.1	0.5
20	zinc	mg/kg	0.058	4	50
21	chloride	mg/kg	63	800	15,000
22	fluoride	mg/kg	2.6	10	150
23	sulphate	mg/kg	110	1,000	20,000
24	phenol index	mg/kg	<0.3	1	-
25	DOC (dissolved organic carbon)	mg/kg	<50	500	800
26	TDS (total dissolved solids)	mg/kg	1100	4,000	60,000

Key

User supplied data

Classification of sample: TP01-1-22/12/2022-0.5

✔ **Non Hazardous Waste**
Classified as **17 05 04**
in the List of Waste

Sample details

Sample name:	LoW Code:
TP01-1-22/12/2022-0.5	Chapter:
Sample Depth:	17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
0.5 m	Entry:
Moisture content:	17 05 04 (Soil and stones other than those mentioned in 17 05 03)
9.5%	
(wet weight correction)	

Hazard properties

None identified

Determinands

Moisture content: 9.5% Wet Weight Moisture Correction applied (MC)

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
1	antimony { antimony trioxide }				2.1 mg/kg	1.197	2.275 mg/kg	0.000228 %	✓	
	051-005-00-X	215-175-0	1309-64-4							
2	arsenic { arsenic trioxide }				12 mg/kg	1.32	14.339 mg/kg	0.00143 %	✓	
	033-003-00-0	215-481-4	1327-53-3							
3	cadmium { cadmium oxide }				0.79 mg/kg	1.142	0.817 mg/kg	0.0000817 %	✓	
	048-002-00-0	215-146-2	1306-19-0							
4	chromium in chromium(III) compounds { chromium(III) oxide (worst case) }				4.1 mg/kg	1.462	5.423 mg/kg	0.000542 %	✓	
		215-160-9	1308-38-9							
5	chromium in chromium(VI) compounds { chromium(VI) compounds, with the exception of barium chromate and of compounds specified elsewhere in this Annex }				<0.5 mg/kg	2.27	<1.135 mg/kg	<0.000113 %		<LOD
	024-017-00-8									
6	copper { dicopper oxide; copper (I) oxide }				30 mg/kg	1.126	30.568 mg/kg	0.00306 %	✓	
	029-002-00-X	215-270-7	1317-39-1							
7	lead { lead chromate }			1	50 mg/kg	1.56	70.582 mg/kg	0.00453 %	✓	
	082-004-00-2	231-846-0	7758-97-6							
8	mercury { mercury dichloride }				0.11 mg/kg	1.353	0.135 mg/kg	0.0000135 %	✓	
	080-010-00-X	231-299-8	7487-94-7							
9	molybdenum { molybdenum(VI) oxide }				2.1 mg/kg	1.5	2.851 mg/kg	0.000285 %	✓	
	042-001-00-9	215-204-7	1313-27-5							
10	nickel { nickel chromate }				11 mg/kg	2.976	29.629 mg/kg	0.00296 %	✓	
	028-035-00-7	238-766-5	14721-18-7							
11	selenium { nickel selenate }				0.46 mg/kg	2.554	1.063 mg/kg	0.000106 %	✓	
	028-031-00-5	239-125-2	15060-62-5							
12	zinc { zinc chromate }				140 mg/kg	2.774	351.484 mg/kg	0.0351 %	✓	
	024-007-00-3	236-878-9	13530-65-9							
13	tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	603-181-00-X	216-653-1	1634-04-4							
14	benzene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	601-020-00-8	200-753-7	71-43-2							
15	toluene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	601-021-00-3	203-625-9	108-88-3							

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
16	ethylbenzene 601-023-00-4	202-849-4	100-41-4		<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
17	xylene 601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<0.002 mg/kg		<0.002 mg/kg	<0.0000002 %		<LOD
18	naphthalene 601-052-00-2	202-049-5	91-20-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
19	acenaphthylene 205-917-1	208-96-8			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
20	acenaphthene 201-469-6	83-32-9			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
21	fluorene 201-695-5	86-73-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
22	phenanthrene 201-581-5	85-01-8			0.19 mg/kg		0.172 mg/kg	0.0000172 %	✓	
23	anthracene 204-371-1	120-12-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
24	fluoranthene 205-912-4	206-44-0			0.34 mg/kg		0.308 mg/kg	0.0000308 %	✓	
25	pyrene 204-927-3	129-00-0			0.37 mg/kg		0.335 mg/kg	0.0000335 %	✓	
26	benzo[a]anthracene 601-033-00-9	200-280-6	56-55-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
27	chrysene 601-048-00-0	205-923-4	218-01-9		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
28	benzo[b]fluoranthene 601-034-00-4	205-911-9	205-99-2		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
29	benzo[k]fluoranthene 601-036-00-5	205-916-6	207-08-9		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
30	benzo[a]pyrene; benzo[def]chrysene 601-032-00-3	200-028-5	50-32-8		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
31	indeno[123-cd]pyrene 205-893-2	193-39-5			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
32	dibenz[a,h]anthracene 601-041-00-2	200-181-8	53-70-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
33	benzo[ghi]perylene 205-883-8	191-24-2			0.18 mg/kg		0.163 mg/kg	0.0000163 %	✓	
34	polychlorobiphenyls; PCB 602-039-00-4	215-648-1	1336-36-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
35	barium { barium sulphide } 016-002-00-X	244-214-4	21109-95-5		48 mg/kg	1.233	53.583 mg/kg	0.00536 %	✓	
36	coronene 205-881-7	191-07-1			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
Total:								0.0541 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
●	Determinand defined or amended by HazWasteOnline (see Appendix A)
⚗	Speciated Determinand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
ND	Not detected
CLP: Note 1	Only the metal concentration has been used for classification

WAC results for sample: TP01-1-22/12/2022-0.5

WAC Settings: samples in this Job do not constitute a single population.

WAC limits used to evaluate this sample: "Ireland"

The WAC used in this report are the WAC defined for the inert and non-hazardous classes of landfill in the Republic of Ireland. You should check the actual acceptance criteria when the disposal site is identified as they may differ from the generic WAC used in this report.

The sample **PASSES** the Inert (Inert waste landfill) criteria.

The sample **PASSES** the Non Haz (Non hazardous waste landfill) criteria.

WAC Determinands

Solid Waste Analysis				Landfill Waste Acceptance Criteria Limits	
#	Determinand		User entered data	Inert waste landfill	Non hazardous waste landfill
1	TOC (total organic carbon)	%	1.2	3	5
2	LOI (loss on ignition)	%	9.7	-	-
3	BTEX (benzene, toluene, ethylbenzene and xylenes)	mg/kg	<0.01	6	-
4	PCBs (polychlorinated biphenyls, 7 congeners)	mg/kg	<0.1	1	-
5	Mineral oil (C10 to C40)	mg/kg	<10	500	-
6	PAHs (polycyclic aromatic hydrocarbons)	mg/kg	<2	100	-
7	pH	pH	8.6	-	>6
8	ANC (acid neutralisation capacity)	mol/kg	0.026	-	-
Eluate Analysis 10:1					
9	arsenic	mg/kg	0.038	0.5	2
10	barium	mg/kg	0.052	20	100
11	cadmium	mg/kg	<0.0011	0.04	1
12	chromium	mg/kg	0.066	0.5	10
13	copper	mg/kg	0.012	2	50
14	mercury	mg/kg	<0.0005	0.01	0.2
15	molybdenum	mg/kg	0.036	0.5	10
16	nickel	mg/kg	<0.005	0.4	10
17	lead	mg/kg	<0.005	0.5	10
18	antimony	mg/kg	0.018	0.06	0.7
19	selenium	mg/kg	0.015	0.1	0.5
20	zinc	mg/kg	<0.025	4	50
21	chloride	mg/kg	<10	800	15,000
22	fluoride	mg/kg	1.2	10	150
23	sulphate	mg/kg	45	1,000	20,000
24	phenol index	mg/kg	<0.3	1	-
25	DOC (dissolved organic carbon)	mg/kg	<50	500	800
26	TDS (total dissolved solids)	mg/kg	520	4,000	60,000

Key

User supplied data

Classification of sample: TP01-2-22/12/2022-1.5

 **Non Hazardous Waste**
 Classified as **17 05 04**
 in the List of Waste

Sample details

Sample name:	LoW Code:
TP01-2-22/12/2022-1.5	Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Sample Depth:	Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)
1.5 m	
Moisture content:	
19%	
(wet weight correction)	

Hazard properties

None identified

Determinands

Moisture content: 19% Wet Weight Moisture Correction applied (MC)

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number								
1	antimony { antimony trioxide }				<2 mg/kg	1.197	<2.394 mg/kg	<0.000239 %			<LOD
	051-005-00-X	215-175-0	1309-64-4								
2	arsenic { arsenic trioxide }				11 mg/kg	1.32	11.764 mg/kg	0.00118 %		✓	
	033-003-00-0	215-481-4	1327-53-3								
3	cadmium { cadmium oxide }				1.3 mg/kg	1.142	1.203 mg/kg	0.00012 %		✓	
	048-002-00-0	215-146-2	1306-19-0								
4	chromium in chromium(III) compounds { chromium(III) oxide (worst case) }				12 mg/kg	1.462	14.206 mg/kg	0.00142 %		✓	
		215-160-9	1308-38-9								
5	chromium in chromium(VI) compounds { chromium(VI) compounds, with the exception of barium chromate and of compounds specified elsewhere in this Annex }				<0.5 mg/kg	2.27	<1.135 mg/kg	<0.000113 %			<LOD
	024-017-00-8										
6	copper { dicopper oxide; copper (I) oxide }				28 mg/kg	1.126	25.535 mg/kg	0.00255 %		✓	
	029-002-00-X	215-270-7	1317-39-1								
7	lead { lead chromate }			1	66 mg/kg	1.56	83.388 mg/kg	0.00535 %		✓	
	082-004-00-2	231-846-0	7758-97-6								
8	mercury { mercury dichloride }				0.36 mg/kg	1.353	0.395 mg/kg	0.0000395 %		✓	
	080-010-00-X	231-299-8	7487-94-7								
9	molybdenum { molybdenum(VI) oxide }				2.8 mg/kg	1.5	3.402 mg/kg	0.00034 %		✓	
	042-001-00-9	215-204-7	1313-27-5								
10	nickel { nickel chromate }				27 mg/kg	2.976	65.091 mg/kg	0.00651 %		✓	
	028-035-00-7	238-766-5	14721-18-7								
11	selenium { nickel selenate }				1.1 mg/kg	2.554	2.275 mg/kg	0.000228 %		✓	
	028-031-00-5	239-125-2	15060-62-5								
12	zinc { zinc chromate }				95 mg/kg	2.774	213.471 mg/kg	0.0213 %		✓	
	024-007-00-3	236-878-9	13530-65-9								
13	tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
	603-181-00-X	216-653-1	1634-04-4								
14	benzene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
	601-020-00-8	200-753-7	71-43-2								
15	toluene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
	601-021-00-3	203-625-9	108-88-3								

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
16	ethylbenzene 601-023-00-4	202-849-4	100-41-4		<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
17	xylene 601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<0.002 mg/kg		<0.002 mg/kg	<0.0000002 %		<LOD
18	naphthalene 601-052-00-2	202-049-5	91-20-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
19	acenaphthylene 205-917-1	208-96-8			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
20	acenaphthene 201-469-6	83-32-9			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
21	fluorene 201-695-5	86-73-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
22	phenanthrene 201-581-5	85-01-8			0.23 mg/kg		0.186 mg/kg	0.0000186 %	✓	
23	anthracene 204-371-1	120-12-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
24	fluoranthene 205-912-4	206-44-0			0.24 mg/kg		0.194 mg/kg	0.0000194 %	✓	
25	pyrene 204-927-3	129-00-0			0.26 mg/kg		0.211 mg/kg	0.0000211 %	✓	
26	benzo[a]anthracene 601-033-00-9	200-280-6	56-55-3		0.14 mg/kg		0.113 mg/kg	0.0000113 %	✓	
27	chrysene 601-048-00-0	205-923-4	218-01-9		0.12 mg/kg		0.0972 mg/kg	0.00000972 %	✓	
28	benzo[b]fluoranthene 601-034-00-4	205-911-9	205-99-2		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
29	benzo[k]fluoranthene 601-036-00-5	205-916-6	207-08-9		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
30	benzo[a]pyrene; benzo[def]chrysene 601-032-00-3	200-028-5	50-32-8		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
31	indeno[123-cd]pyrene 205-893-2	193-39-5			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
32	dibenz[a,h]anthracene 601-041-00-2	200-181-8	53-70-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
33	benzo[ghi]perylene 205-883-8	191-24-2			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
34	polychlorobiphenyls; PCB 602-039-00-4	215-648-1	1336-36-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
35	barium { barium sulphide } 016-002-00-X	244-214-4	21109-95-5		68 mg/kg	1.233	67.941 mg/kg	0.00679 %	✓	
36	coronene 205-881-7	191-07-1			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %		<LOD
Total:								0.0464 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
•	Determinand defined or amended by HazWasteOnline (see Appendix A)
•	Speciated Determinand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
ND	Not detected
CLP: Note 1	Only the metal concentration has been used for classification

WAC results for sample: TP01-2-22/12/2022-1.5

WAC Settings: samples in this Job do not constitute a single population.

WAC limits used to evaluate this sample: "Ireland"

The WAC used in this report are the WAC defined for the inert and non-hazardous classes of landfill in the Republic of Ireland. You should check the actual acceptance criteria when the disposal site is identified as they may differ from the generic WAC used in this report.

The sample FAILS the Inert (Inert waste landfill) criteria.

The sample PASSES the Non Haz (Non hazardous waste landfill) criteria.

WAC Determinands

Solid Waste Analysis				Landfill Waste Acceptance Criteria Limits	
#	Determinand		User entered data	Inert waste landfill	Non hazardous waste landfill
1	TOC (total organic carbon)	%	3.3	3	5
2	LOI (loss on ignition)	%	3.8	-	-
3	BTEX (benzene, toluene, ethylbenzene and xylenes)	mg/kg	<0.01	6	-
4	PCBs (polychlorinated biphenyls, 7 congeners)	mg/kg	<0.1	1	-
5	Mineral oil (C10 to C40)	mg/kg	<10	500	-
6	PAHs (polycyclic aromatic hydrocarbons)	mg/kg	<2	100	-
7	pH	pH	8.2	-	>6
8	ANC (acid neutralisation capacity)	mol/kg	0.025	-	-
Eluate Analysis 10:1					
9	arsenic	mg/kg	0.012	0.5	2
10	barium	mg/kg	0.079	20	100
11	cadmium	mg/kg	<0.0011	0.04	1
12	chromium	mg/kg	<0.005	0.5	10
13	copper	mg/kg	0.011	2	50
14	mercury	mg/kg	<0.0005	0.01	0.2
15	molybdenum	mg/kg	0.13	0.5	10
16	nickel	mg/kg	0.0065	0.4	10
17	lead	mg/kg	<0.005	0.5	10
18	antimony	mg/kg	0.012	0.06	0.7
19	selenium	mg/kg	0.0098	0.1	0.5
20	zinc	mg/kg	<0.025	4	50
21	chloride	mg/kg	<10	800	15,000
22	fluoride	mg/kg	2.3	10	150
23	sulphate	mg/kg	16	1,000	20,000
24	phenol index	mg/kg	<0.3	1	-
25	DOC (dissolved organic carbon)	mg/kg	<50	500	800
26	TDS (total dissolved solids)	mg/kg	1100	4,000	60,000

Key

	User supplied data
	Inert WAC criteria fail

Classification of sample: TP02-1-22/12/2022-0.5

✔ **Non Hazardous Waste**
Classified as **17 05 04**
in the List of Waste

Sample details

Sample name:	LoW Code:
TP02-1-22/12/2022-0.5	Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Sample Depth:	Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)
0.5 m	
Moisture content:	
17%	
(wet weight correction)	

Hazard properties

None identified

Determinands

Moisture content: 17% Wet Weight Moisture Correction applied (MC)

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number							
1	antimony { antimony trioxide }				<2 mg/kg	1.197	<2.394 mg/kg	<0.000239 %		<LOD
	051-005-00-X	215-175-0	1309-64-4							
2	arsenic { arsenic trioxide }				8.7 mg/kg	1.32	9.534 mg/kg	0.000953 %	✓	
	033-003-00-0	215-481-4	1327-53-3							
3	cadmium { cadmium oxide }				0.45 mg/kg	1.142	0.427 mg/kg	0.0000427 %	✓	
	048-002-00-0	215-146-2	1306-19-0							
4	chromium in chromium(III) compounds { chromium(III) oxide (worst case) }				8.3 mg/kg	1.462	10.069 mg/kg	0.00101 %	✓	
		215-160-9	1308-38-9							
5	chromium in chromium(VI) compounds { chromium(VI) compounds, with the exception of barium chromate and of compounds specified elsewhere in this Annex }				<0.5 mg/kg	2.27	<1.135 mg/kg	<0.000113 %		<LOD
	024-017-00-8									
6	copper { dicopper oxide; copper (I) oxide }				33 mg/kg	1.126	30.838 mg/kg	0.00308 %	✓	
	029-002-00-X	215-270-7	1317-39-1							
7	lead { lead chromate }			1	260 mg/kg	1.56	336.608 mg/kg	0.0216 %	✓	
	082-004-00-2	231-846-0	7758-97-6							
8	mercury { mercury dichloride }				1.6 mg/kg	1.353	1.797 mg/kg	0.00018 %	✓	
	080-010-00-X	231-299-8	7487-94-7							
9	molybdenum { molybdenum(VI) oxide }				0.7 mg/kg	1.5	0.872 mg/kg	0.0000872 %	✓	
	042-001-00-9	215-204-7	1313-27-5							
10	nickel { nickel chromate }				11 mg/kg	2.976	27.173 mg/kg	0.00272 %	✓	
	028-035-00-7	238-766-5	14721-18-7							
11	selenium { nickel selenate }				0.25 mg/kg	2.554	0.53 mg/kg	0.000053 %	✓	
	028-031-00-5	239-125-2	15060-62-5							
12	zinc { zinc chromate }				77 mg/kg	2.774	177.296 mg/kg	0.0177 %	✓	
	024-007-00-3	236-878-9	13530-65-9							
13	tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	603-181-00-X	216-653-1	1634-04-4							
14	benzene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	601-020-00-8	200-753-7	71-43-2							
15	toluene				<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %		<LOD
	601-021-00-3	203-625-9	108-88-3							

#	Determinand			CLP Note	User entered data	Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
	EU CLP index number	EC Number	CAS Number								
16	ethylbenzene 601-023-00-4	202-849-4	100-41-4		<0.001 mg/kg		<0.001 mg/kg	<0.0000001 %			<LOD
17	xylene 601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<0.002 mg/kg		<0.002 mg/kg	<0.0000002 %			<LOD
18	naphthalene 601-052-00-2	202-049-5	91-20-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
19	acenaphthylene 205-917-1	208-96-8			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
20	acenaphthene 201-469-6	83-32-9			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
21	fluorene 201-695-5	86-73-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
22	phenanthrene 201-581-5	85-01-8			0.16 mg/kg		0.133 mg/kg	0.0000133 %	✓		
23	anthracene 204-371-1	120-12-7			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
24	fluoranthene 205-912-4	206-44-0			0.18 mg/kg		0.149 mg/kg	0.0000149 %	✓		
25	pyrene 204-927-3	129-00-0			0.17 mg/kg		0.141 mg/kg	0.0000141 %	✓		
26	benzo[a]anthracene 601-033-00-9	200-280-6	56-55-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
27	chrysene 601-048-00-0	205-923-4	218-01-9		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
28	benzo[b]fluoranthene 601-034-00-4	205-911-9	205-99-2		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
29	benzo[k]fluoranthene 601-036-00-5	205-916-6	207-08-9		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
30	benzo[a]pyrene; benzo[def]chrysene 601-032-00-3	200-028-5	50-32-8		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
31	indeno[123-cd]pyrene 205-893-2	193-39-5			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
32	dibenz[a,h]anthracene 601-041-00-2	200-181-8	53-70-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
33	benzo[ghi]perylene 205-883-8	191-24-2			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
34	polychlorobiphenyls; PCB 602-039-00-4	215-648-1	1336-36-3		<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
35	barium { barium sulphide } 016-002-00-X	244-214-4	21109-95-5		150 mg/kg	1.233	153.57 mg/kg	0.0154 %	✓		
36	coronene 205-881-7	191-07-1			<0.1 mg/kg		<0.1 mg/kg	<0.00001 %			<LOD
Total:									0.0633 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
●	Determinand defined or amended by HazWasteOnline (see Appendix A)
⚗	Speciated Determinand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
ND	Not detected
CLP: Note 1	Only the metal concentration has been used for classification

WAC results for sample: TP02-1-22/12/2022-0.5

WAC Settings: samples in this Job do not constitute a single population.

WAC limits used to evaluate this sample: "Ireland"

The WAC used in this report are the WAC defined for the inert and non-hazardous classes of landfill in the Republic of Ireland. You should check the actual acceptance criteria when the disposal site is identified as they may differ from the generic WAC used in this report.

The sample **PASSES** the Inert (Inert waste landfill) criteria.

The sample **PASSES** the Non Haz (Non hazardous waste landfill) criteria.

WAC Determinands

Solid Waste Analysis				Landfill Waste Acceptance Criteria Limits	
#	Determinand		User entered data	Inert waste landfill	Non hazardous waste landfill
1	TOC (total organic carbon)	%	0.91	3	5
2	LOI (loss on ignition)	%	4.3	-	-
3	BTEX (benzene, toluene, ethylbenzene and xylenes)	mg/kg	<0.01	6	-
4	PCBs (polychlorinated biphenyls, 7 congeners)	mg/kg	<0.1	1	-
5	Mineral oil (C10 to C40)	mg/kg	<10	500	-
6	PAHs (polycyclic aromatic hydrocarbons)	mg/kg	<2	100	-
7	pH	pH	8.3	-	>6
8	ANC (acid neutralisation capacity)	mol/kg	0.029	-	-
Eluate Analysis 10:1					
9	arsenic	mg/kg	0.3	0.5	2
10	barium	mg/kg	0.32	20	100
11	cadmium	mg/kg	<0.0011	0.04	1
12	chromium	mg/kg	0.47	0.5	10
13	copper	mg/kg	0.011	2	50
14	mercury	mg/kg	<0.0005	0.01	0.2
15	molybdenum	mg/kg	0.012	0.5	10
16	nickel	mg/kg	<0.005	0.4	10
17	lead	mg/kg	0.037	0.5	10
18	antimony	mg/kg	0.015	0.06	0.7
19	selenium	mg/kg	0.0098	0.1	0.5
20	zinc	mg/kg	0.073	4	50
21	chloride	mg/kg	<10	800	15,000
22	fluoride	mg/kg	4.2	10	150
23	sulphate	mg/kg	64	1,000	20,000
24	phenol index	mg/kg	<0.3	1	-
25	DOC (dissolved organic carbon)	mg/kg	<50	500	800
26	TDS (total dissolved solids)	mg/kg	650	4,000	60,000

Key

User supplied data

Appendix A: Classifier defined and non EU CLP determinands

■ chromium(III) oxide (worst case) (EC Number: 215-160-9, CAS Number: 1308-38-9)

Description/Comments: Data from C&L Inventory Database

Data source: <https://echa.europa.eu/information-on-chemicals/cl-inventory-database/-/discli/details/33806>

Data source date: 17 Jul 2015

Hazard Statements: Acute Tox. 4; H332, Acute Tox. 4; H302, Eye Irrit. 2; H319, STOT SE 3; H335, Skin Irrit. 2; H315, Resp. Sens. 1; H334, Skin Sens. 1; H317, Repr. 1B; H360FD, Aquatic Acute 1; H400, Aquatic Chronic 1; H410

■ ethylbenzene (EC Number: 202-849-4, CAS Number: 100-41-4)

EU CLP index number: 601-023-00-4

Description/Comments:

Additional Hazard Statement(s): Carc. 2; H351

Reason for additional Hazards Statement(s):

03 Jun 2015 - Carc. 2; H351 hazard statement sourced from: IARC Group 2B (77) 2000

■ acenaphthylene (EC Number: 205-917-1, CAS Number: 208-96-8)

Description/Comments: Data from C&L Inventory Database

Data source: <http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database>

Data source date: 17 Jul 2015

Hazard Statements: Acute Tox. 4; H302, Acute Tox. 1; H330, Acute Tox. 1; H310, Eye Irrit. 2; H319, STOT SE 3; H335, Skin Irrit. 2; H315

■ acenaphthene (EC Number: 201-469-6, CAS Number: 83-32-9)

Description/Comments: Data from C&L Inventory Database

Data source: <http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database>

Data source date: 17 Jul 2015

Hazard Statements: Eye Irrit. 2; H319, STOT SE 3; H335, Skin Irrit. 2; H315, Aquatic Acute 1; H400, Aquatic Chronic 1; H410, Aquatic Chronic 2; H411

■ fluorene (EC Number: 201-695-5, CAS Number: 86-73-7)

Description/Comments: Data from C&L Inventory Database

Data source: <http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database>

Data source date: 06 Aug 2015

Hazard Statements: Aquatic Acute 1; H400, Aquatic Chronic 1; H410

■ phenanthrene (EC Number: 201-581-5, CAS Number: 85-01-8)

Description/Comments: Data from C&L Inventory Database

Data source: <http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database>

Data source date: 06 Aug 2015

Hazard Statements: Acute Tox. 4; H302, Eye Irrit. 2; H319, STOT SE 3; H335, Carc. 2; H351, Skin Sens. 1; H317, Aquatic Acute 1; H400, Aquatic Chronic 1; H410, Skin Irrit. 2; H315

■ anthracene (EC Number: 204-371-1, CAS Number: 120-12-7)

Description/Comments: Data from C&L Inventory Database

Data source: <http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database>

Data source date: 17 Jul 2015

Hazard Statements: Eye Irrit. 2; H319, STOT SE 3; H335, Skin Irrit. 2; H315, Skin Sens. 1; H317, Aquatic Acute 1; H400, Aquatic Chronic 1; H410

■ fluoranthene (EC Number: 205-912-4, CAS Number: 206-44-0)

Description/Comments: Data from C&L Inventory Database

Data source: <http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database>

Data source date: 21 Aug 2015

Hazard Statements: Acute Tox. 4; H302, Aquatic Acute 1; H400, Aquatic Chronic 1; H410

■ pyrene (EC Number: 204-927-3, CAS Number: 129-00-0)

Description/Comments: Data from C&L Inventory Database; SDS Sigma Aldrich 2014

Data source: <http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database>

Data source date: 21 Aug 2015

Hazard Statements: Skin Irrit. 2; H315, Eye Irrit. 2; H319, STOT SE 3; H335, Aquatic Acute 1; H400, Aquatic Chronic 1; H410

■ indeno[123-cd]pyrene (EC Number: 205-893-2, CAS Number: 193-39-5)

Description/Comments: Data from C&L Inventory Database

Data source: <http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database>

Data source date: 06 Aug 2015

Hazard Statements: Carc. 2; H351

▪ **benzo[ghi]perylene** (EC Number: 205-883-8, CAS Number: 191-24-2)

Description/Comments: Data from C&L Inventory Database; SDS Sigma Aldrich 28/02/2015
 Data source: <http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database>
 Data source date: 23 Jul 2015
 Hazard Statements: Aquatic Acute 1; H400 , Aquatic Chronic 1; H410

▪ **polychlorobiphenyls; PCB** (EC Number: 215-648-1, CAS Number: 1336-36-3)

EU CLP index number: 602-039-00-4
 Description/Comments: Worst Case: IARC considers PCB Group 1; Carcinogenic to humans; POP specific threshold from ATP1 (Regulation 756/2010/EU) to POPs Regulation (Regulation 850/2004/EC). Where applicable, the calculation method laid down in European standards EN 12766-1 and EN 12766-2 shall be applied.
 Additional Hazard Statement(s): Carc. 1A; H350
 Reason for additional Hazards Statement(s):
 29 Sep 2015 - Carc. 1A; H350 hazard statement sourced from: IARC Group 1 (23, Sup 7, 100C) 2012

▪ **barium sulphide** (EC Number: 244-214-4, CAS Number: 21109-95-5)

EU CLP index number: 016-002-00-X
 Description/Comments:
 Additional Hazard Statement(s): EUH031 >= 0.8 %
 Reason for additional Hazards Statement(s):
 14 Dec 2015 - EUH031 >= 0.8 % hazard statement sourced from: WM3, Table C12.2

▪ **coronene** (EC Number: 205-881-7, CAS Number: 191-07-1)

Description/Comments: Data from C&L Inventory Database; no entries in Registered Substances or Pesticides Properties databases; SDS: Sigma Aldrich, 1907/2006 compliant, dated 2012 - no entries; IARC – Group 3, not carcinogenic.
 Data source: <http://clp-inventory.echa.europa.eu/SummaryOfClassAndLabelling.aspx?SubstanceID=17010&HarmOnly=no?fc=true&lang=en>
 Data source date: 16 Jun 2014
 Hazard Statements: STOT SE 2; H371

Appendix B: Rationale for selection of metal species

antimony {antimony trioxide}

Worst case CLP species based on hazard statements/molecular weight and low solubility. Industrial sources include: flame retardants in electrical apparatus, textiles and coatings

arsenic {arsenic trioxide}

Reasonable case CLP species based on hazard statements/molecular weight and most common (stable) oxide of arsenic. Industrial sources include: smelting; main precursor to other arsenic compounds

cadmium {cadmium oxide}

Reasonable case CLP species based on hazard statements/molecular weight, very low solubility in water. Industrial sources include: electroplating baths, electrodes for storage batteries, catalysts, ceramic glazes, phosphors, pigments and nematocides.

chromium in chromium(III) compounds {chromium(III) oxide (worst case)}

Reasonable case species based on hazard statements/molecular weight. Industrial sources include: tanning, pigment in paint, inks and glass

chromium in chromium(VI) compounds {chromium (VI) compounds, with the exception of barium chromate and of compounds specified elsewhere in this Annex}

Worst case species based on hazard statements/molecular weight

copper {dicopper oxide; copper (I) oxide}

Reasonable case CLP species based on hazard statements/molecular weight and insolubility in water. Industrial sources include: oxidised copper metal, brake pads, pigments, antifouling paints, fungicide.

lead {lead chromate}

Worst case CLP species based on hazard statements/molecular weight

mercury {mercury dichloride}

Worst case CLP species based on hazard statements/molecular weight (edit as required)

molybdenum {molybdenum(VI) oxide}

Worst case CLP species based on hazard statements/molecular weight

nickel {nickel chromate}

Worst case CLP species based on hazard statements/molecular weight

selenium {nickel selenate}

Worst case CLP species based on hazard statements/molecular weight

zinc {zinc chromate}

Worst case CLP species based on hazard statements/molecular weight

barium {barium sulphide}

No Cr VI in samples therefore worst case scenario not applicable.

Appendix C: Version

HazWasteOnline Classification Engine: **WM3 1st Edition v1.1.NI - Jan 2021**

HazWasteOnline Classification Engine Version: 2023.11.5483.10161 (11 Jan 2023)

HazWasteOnline Database: 2023.11.5483.10161 (11 Jan 2023)

This classification utilises the following guidance and legislation:

WM3 v1.1.NI - Waste Classification - 1st Edition v1.1.NI - Jan 2021

CLP Regulation - Regulation 1272/2008/EC of 16 December 2008

1st ATP - Regulation 790/2009/EC of 10 August 2009

2nd ATP - Regulation 286/2011/EC of 10 March 2011

3rd ATP - Regulation 618/2012/EU of 10 July 2012

4th ATP - Regulation 487/2013/EU of 8 May 2013

Correction to 1st ATP - Regulation 758/2013/EU of 7 August 2013

5th ATP - Regulation 944/2013/EU of 2 October 2013

6th ATP - Regulation 605/2014/EU of 5 June 2014

WFD Annex III replacement - Regulation 1357/2014/EU of 18 December 2014

Revised List of Waste 2014 - Decision 2014/955/EU of 18 December 2014

7th ATP - Regulation 2015/1221/EU of 24 July 2015

8th ATP - Regulation (EU) 2016/918 of 19 May 2016

9th ATP - Regulation (EU) 2016/1179 of 19 July 2016

10th ATP - Regulation (EU) 2017/776 of 4 May 2017

HP14 amendment - Regulation (EU) 2017/997 of 8 June 2017

13th ATP - Regulation (EU) 2018/1480 of 4 October 2018

14th ATP - Regulation (EU) 2020/217 of 4 October 2019

15th ATP - Regulation (EU) 2020/1182 of 19 May 2020

The Chemicals (Health and Safety) and Genetically Modified Organisms (Contained Use)(Amendment etc.) (EU Exit)

Regulations 2020 - UK: 2020 No. 1567 of 16th December 2020

The Waste and Environmental Permitting etc. (Legislative Functions and Amendment etc.) (EU Exit) Regulations 2020 - UK: 2020 No. 1540 of 16th December 2020

17th ATP - Regulation (EU) 2021/849 of 11 March 2021

18th ATP - Regulation (EU) 2022/692 of 16 February 2022



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